



45 Spyglass Drive, Littleton, CO. 80123
Voice: 303.881-8925 Email: normherman@arcodetv.com

ADDENDUM #1

July 27, 2024

**RE: Brakes Plus
2505 Main Street
Norman, OK**

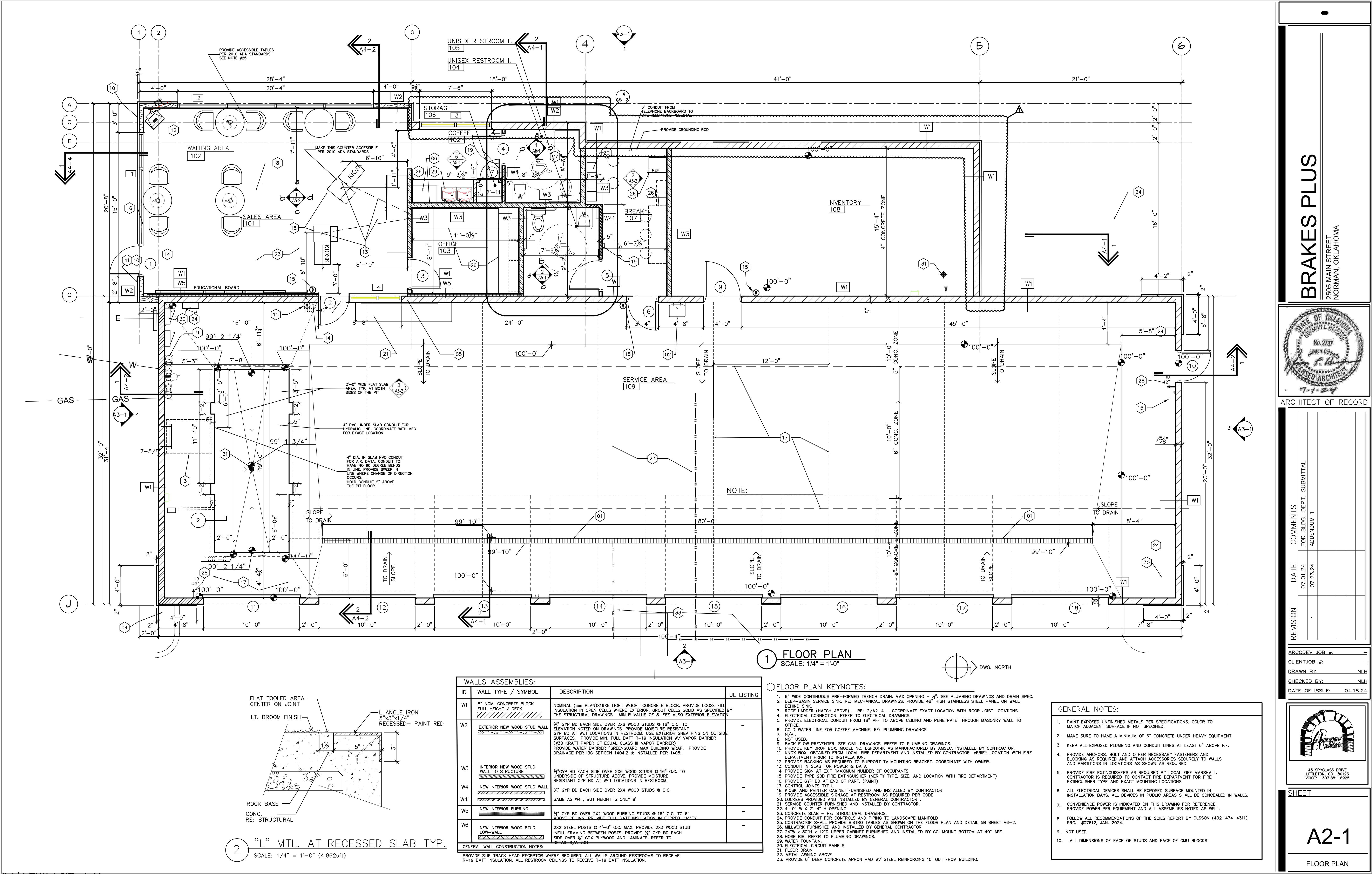
Page 1 of 1

Item No. 1: Wall Type Change:

A portion of the exterior wall surrounding the inventory area are within 3'-0" of the property line. For this reason, they must be 2 hour fire rated. See attached drawings A2-1, A2.2, A2-3, A3-1, A4-1, A4-3, S2-0, S2-1, S3-0, S3.1 and A3-2 For change to 2 hour fire rated walls.

Also see A3-1 for resulting finish changes and wall heights.

END ADDENDUM #1



BRAKES PLUS

2505 MAIN STREET
NORMAN, OKLAHOMA

STATE OF OKLAHOMA
JUDICIAL DISTRICT
No. 2737
J. P. H.
REGISTERED ARCHITECT
7.1.24

ARCHITECT OF RECORD

REVISION	DATE	COMMENTS
1	07.01.24	FOR BLDG. DEPT. SUBMITTAL
	07.23.24	ADDENDUM 1

ARCOCODE JOB #:

CLIENTJOB #:

DRAWN BY: NLH

CHECKED BY: NLH

DATE OF ISSUE: 04.18.24

45 SPYGLASS DRIVE
LITTLETON, CO 80123
VOICE: 303.981-8925

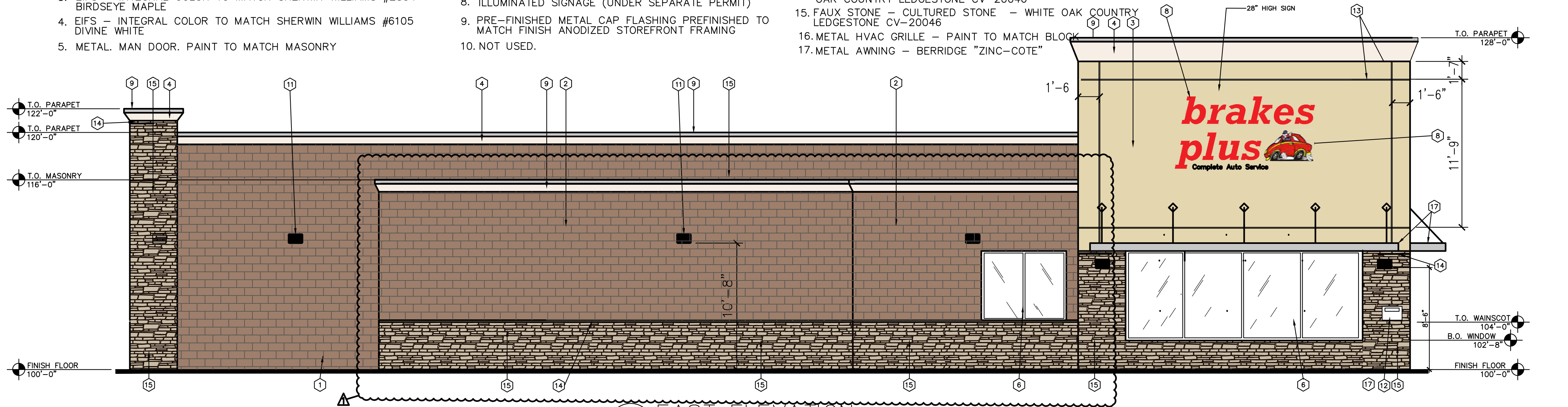
SHEET

A2-1

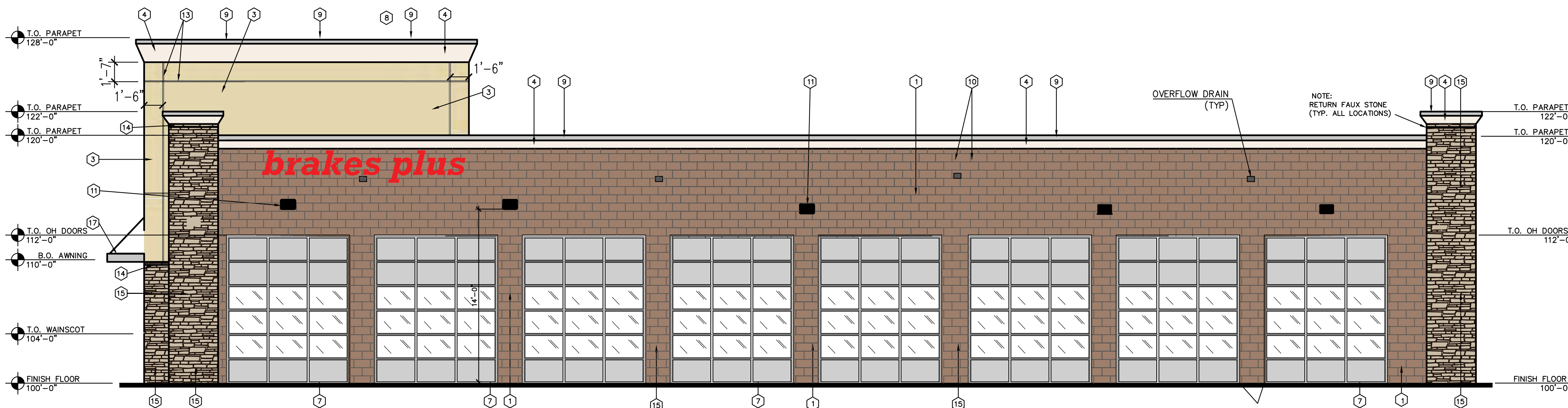
FLOOR PLAN

KEYNOTES

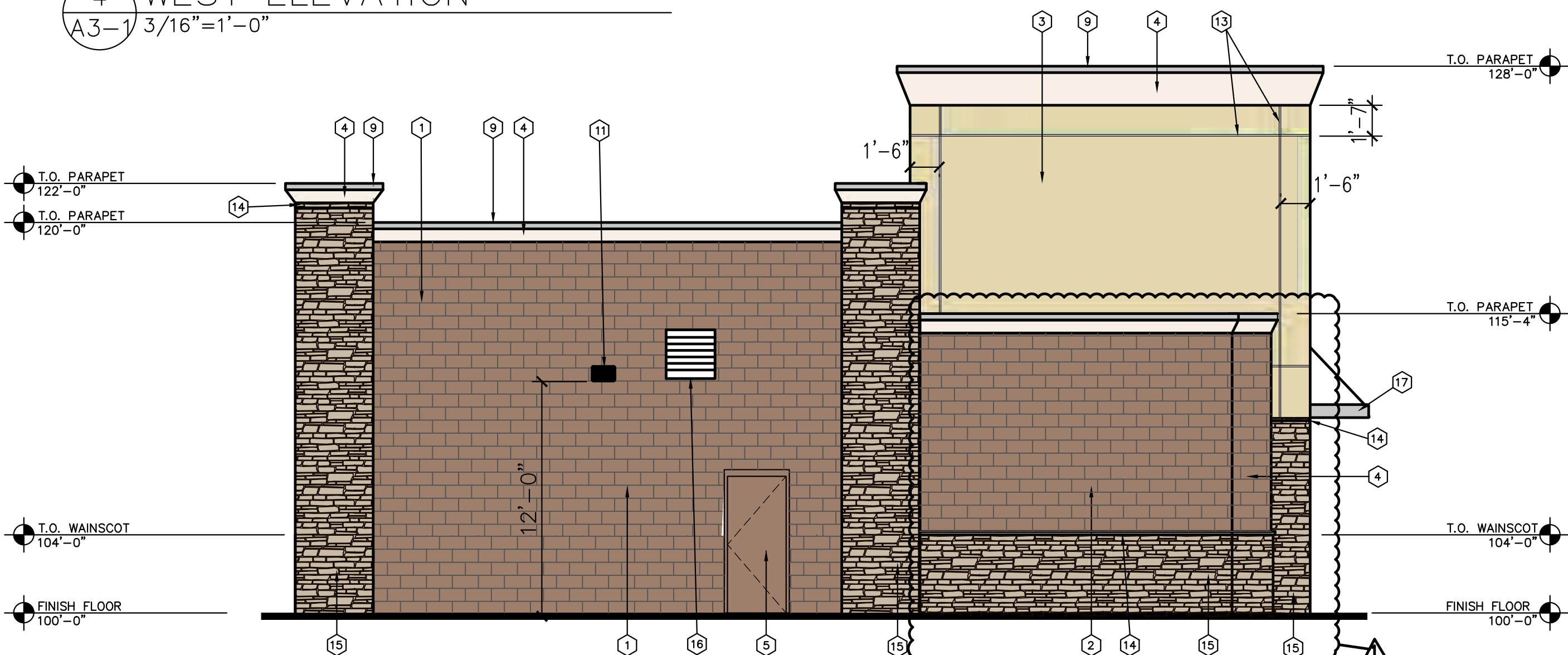
1. SMOOTH FACE CONCRETE BLOCK BY BEST BLOCK
COLOR: #739 MEDIUM BROWN
2. EIFS - INTEGRAL COLOR TO MATCH SHERWIN WILLIAMS #2835
CRAFTSMAN BROWN
3. EIFS - INTEGRAL COLOR TO MATCH SHERWIN WILLIAMS #2834
BIRDSEYE MAPLE
4. EIFS - INTEGRAL COLOR TO MATCH SHERWIN WILLIAMS #6105
DIVINE WHITE
5. METAL MAN DOOR. PAINT TO MATCH MASONRY
6. ALUMINUM/GLASS STOREFRONT
CLEAR ANODIZED ALUMINUM
7. ALUMINUM SECTIONAL OVERHEAD DOORS
CLEAR ANODIZED ALUMINUM
8. ILLUMINATED SIGNAGE (UNDER SEPARATE PERMIT)
9. PRE-FINISHED METAL CAP FLASHING PREFINISHED TO
MATCH FINISH ANODIZED STOREFRONT FRAMING
10. NOT USED.
11. DECORATIVE LIGHT FIXTURE.
12. KEY DROP BOX
13. 1 1/2" X 3/4" REVEAL
14. WATER SILL - FAUX STONE - CULTURED STONE - WHITE
OAK COUNTRY LEDGESTONE CV-20046
15. FAUX STONE - CULTURED STONE - WHITE OAK
LEDGESTONE CV-20046
16. METAL HVAC GRILLE - PAINT TO MATCH BLOCK
17. METAL AWNING - BERRIDGE "ZINC-COTE"



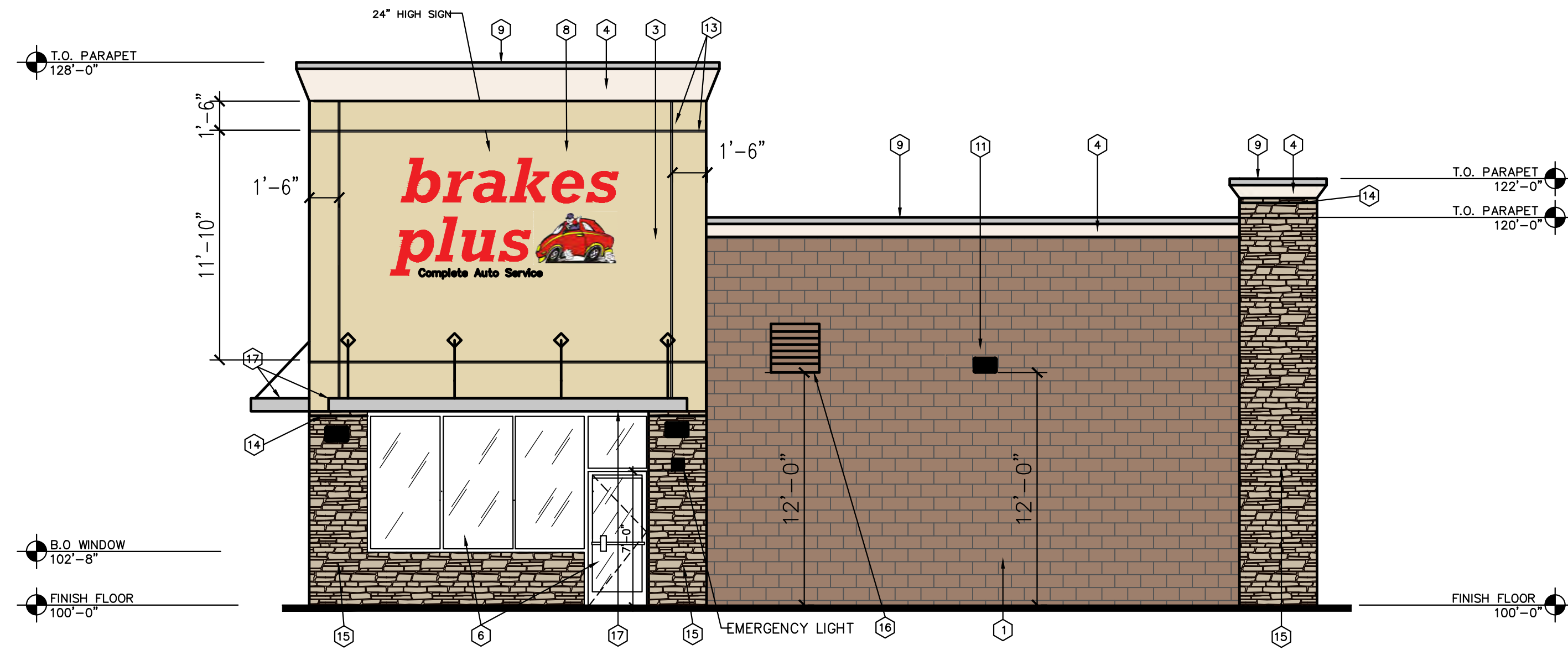
1 EAST ELEVATION
A3-1 3/16"=1'-0"



4 WEST ELEVATION
A3-1 3/16"=1'-0"



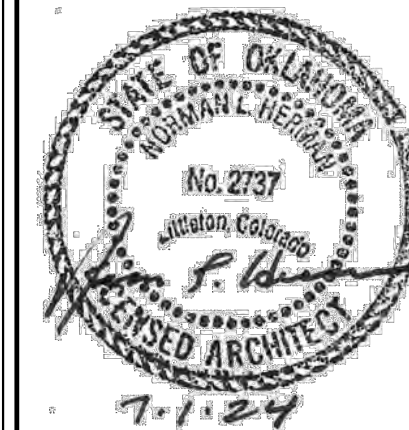
2 NORTH ELEVATION
A3-1 3/16"=1'-0"



3 SOUTH ELEVATION
A3-1 3/16"=1'-0"

BRAKES PLUS

2505 MAIN STREET
NORMAN, OKLAHOMA



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	07.23.24	APPENDUM 1

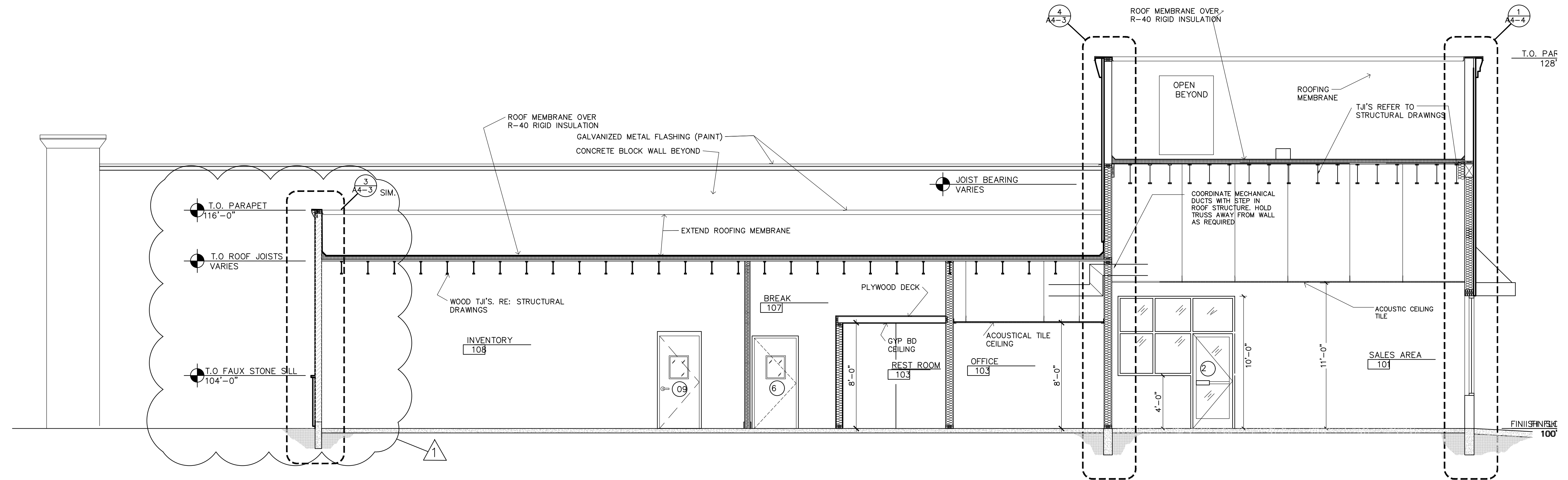
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CLIENTJOB #: -
DRAWN BY: NLH
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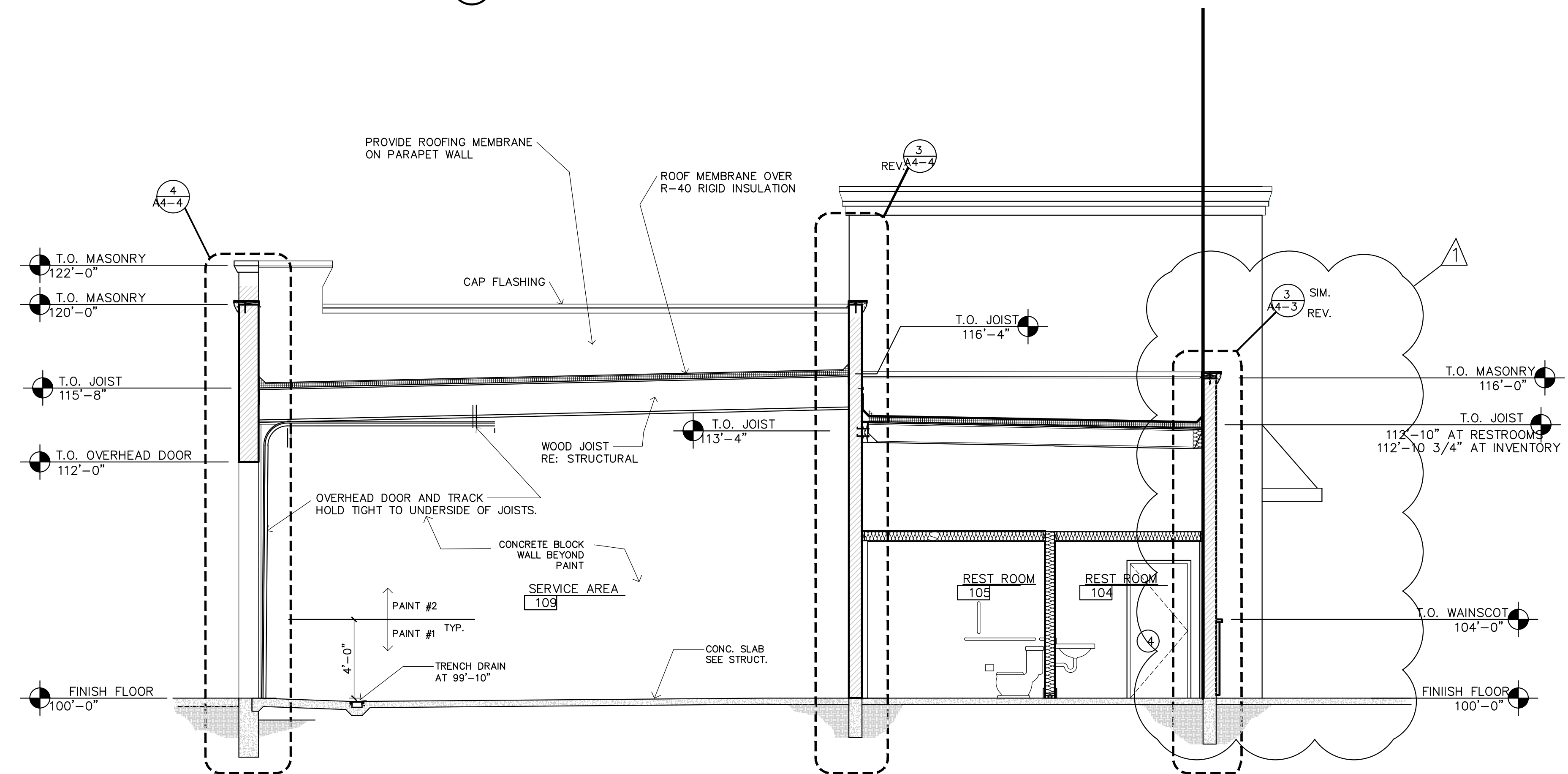
SHEET

A3-1

EXTERIOR ELEVATIONS



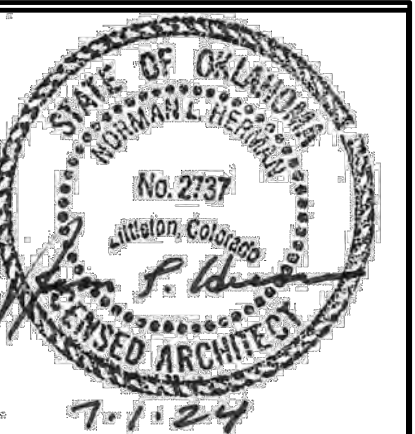
1 BUILDING SECTION
SCALE: 1/4" = 1'-0"



2 BUILDING SECTION
SCALE: 1/4" = 1'-0"

BRAKES PLUS

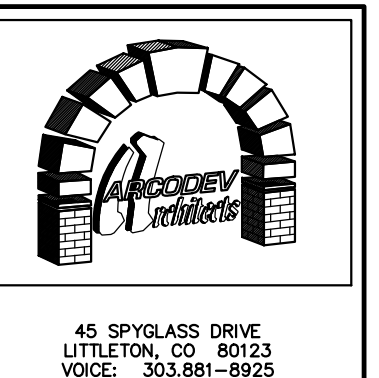
2505 MAIN STREET
NORMAN, OKLAHOMA



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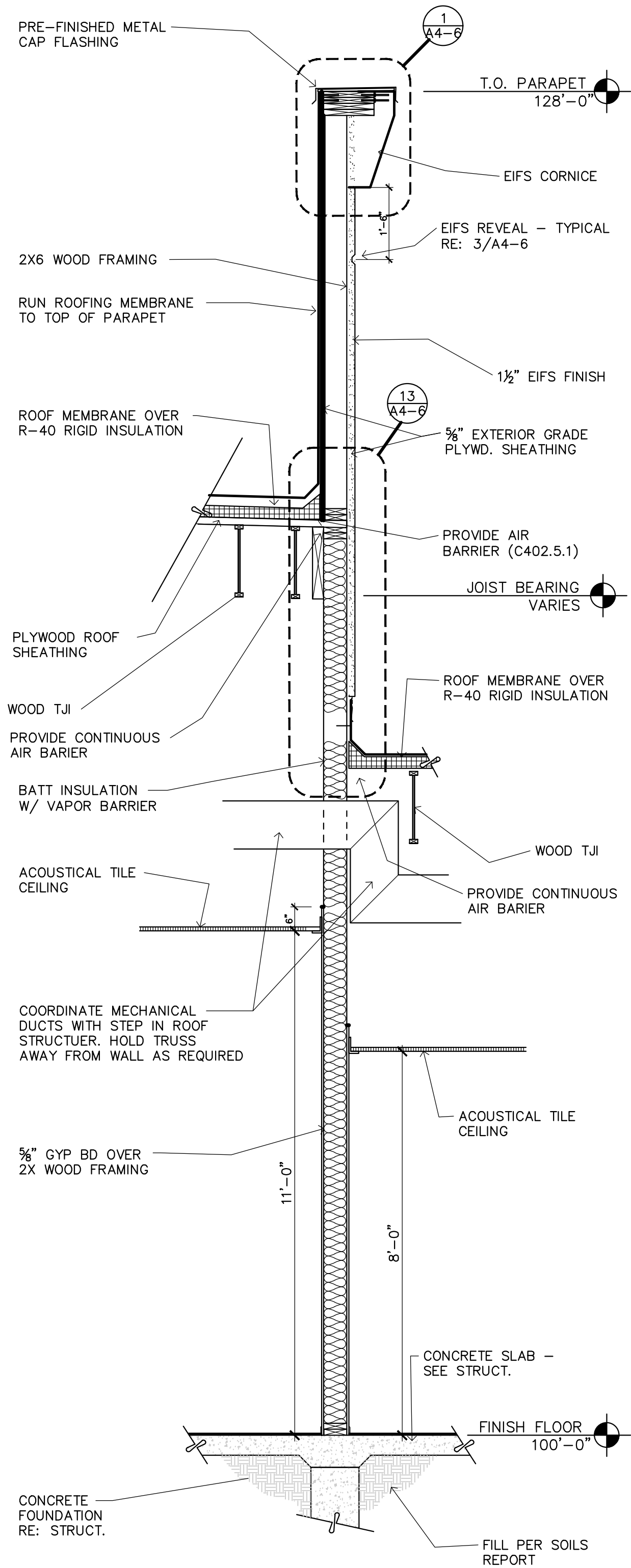
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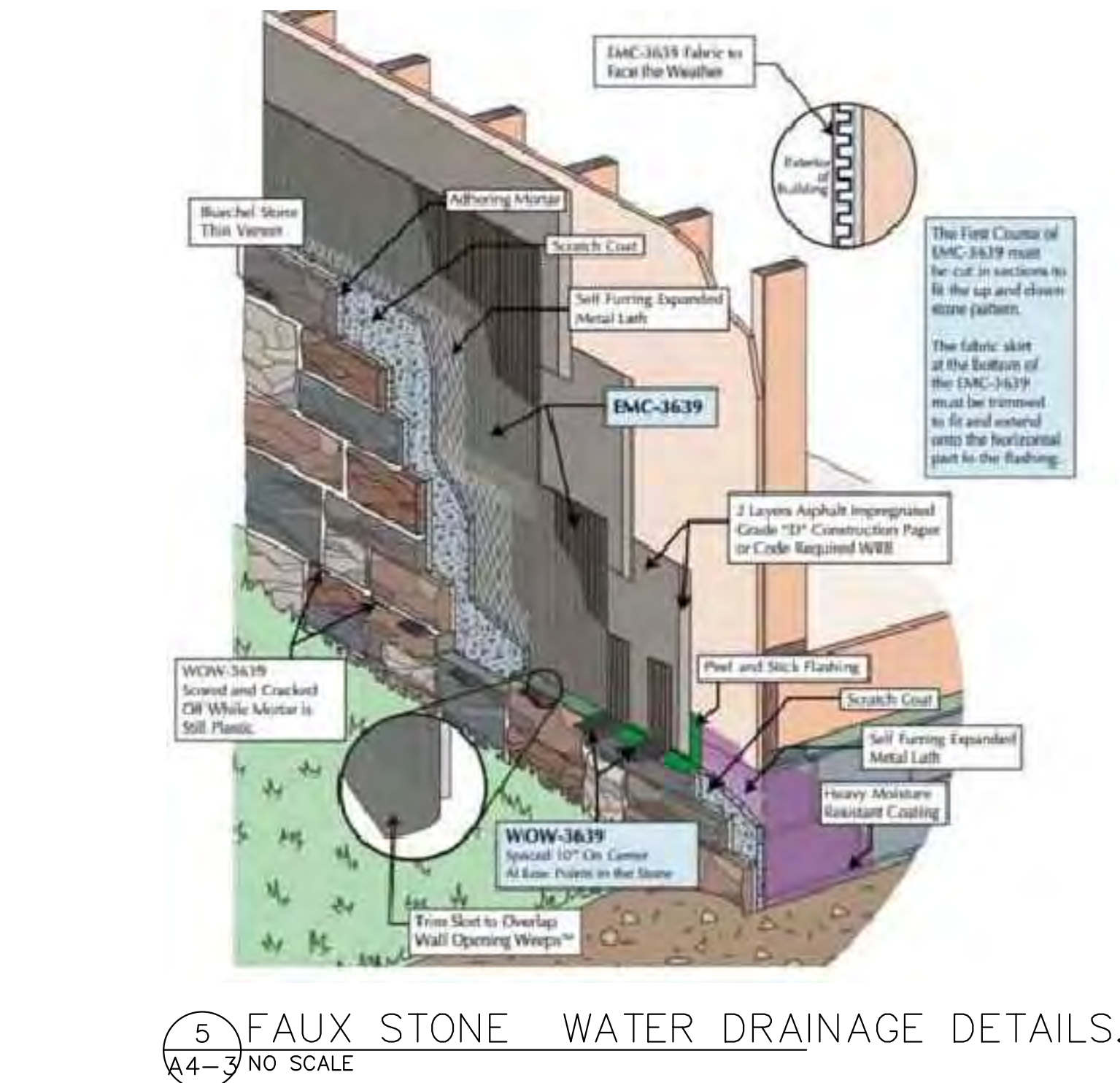
SHEET

A4-1

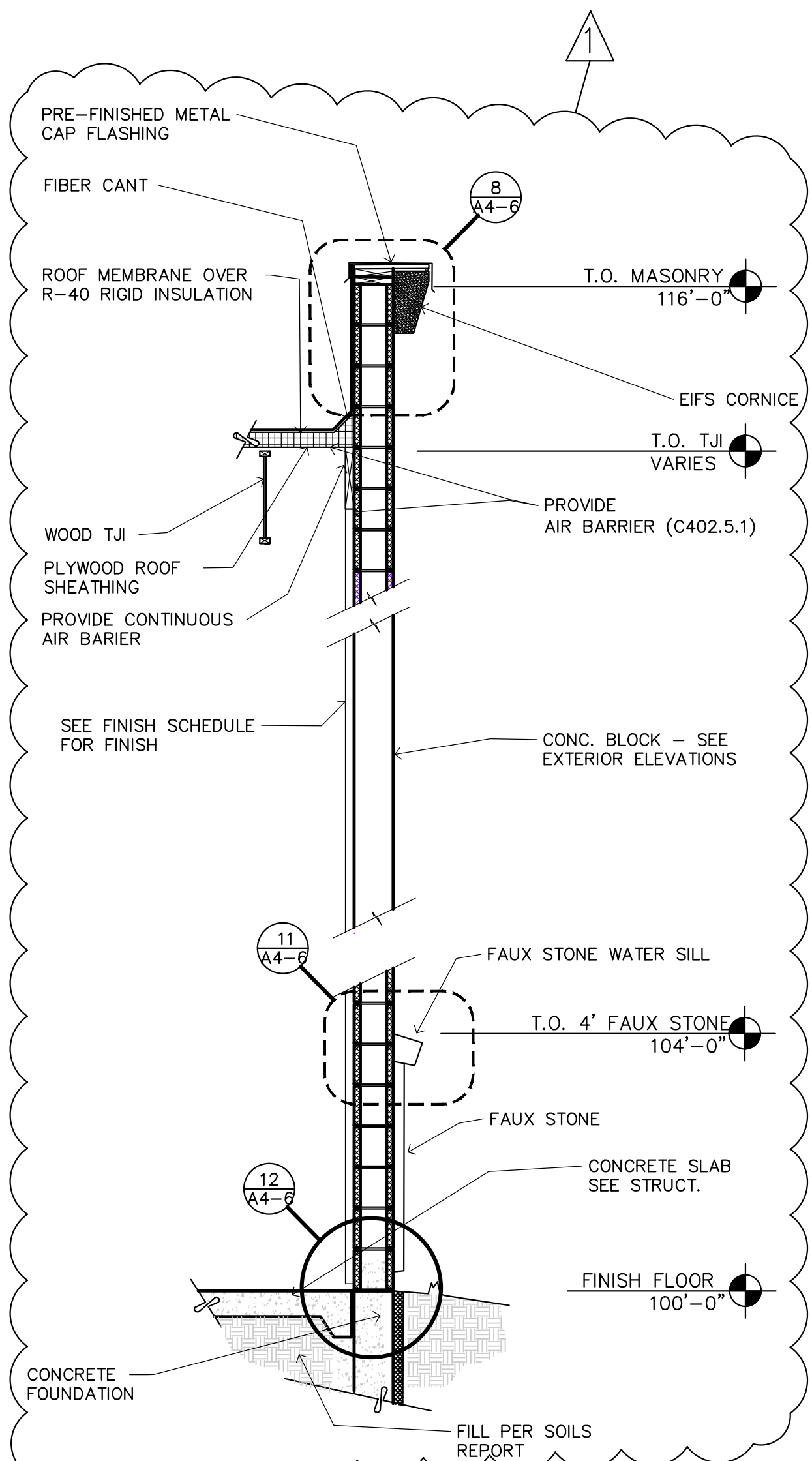
BUILDING SECTIONS



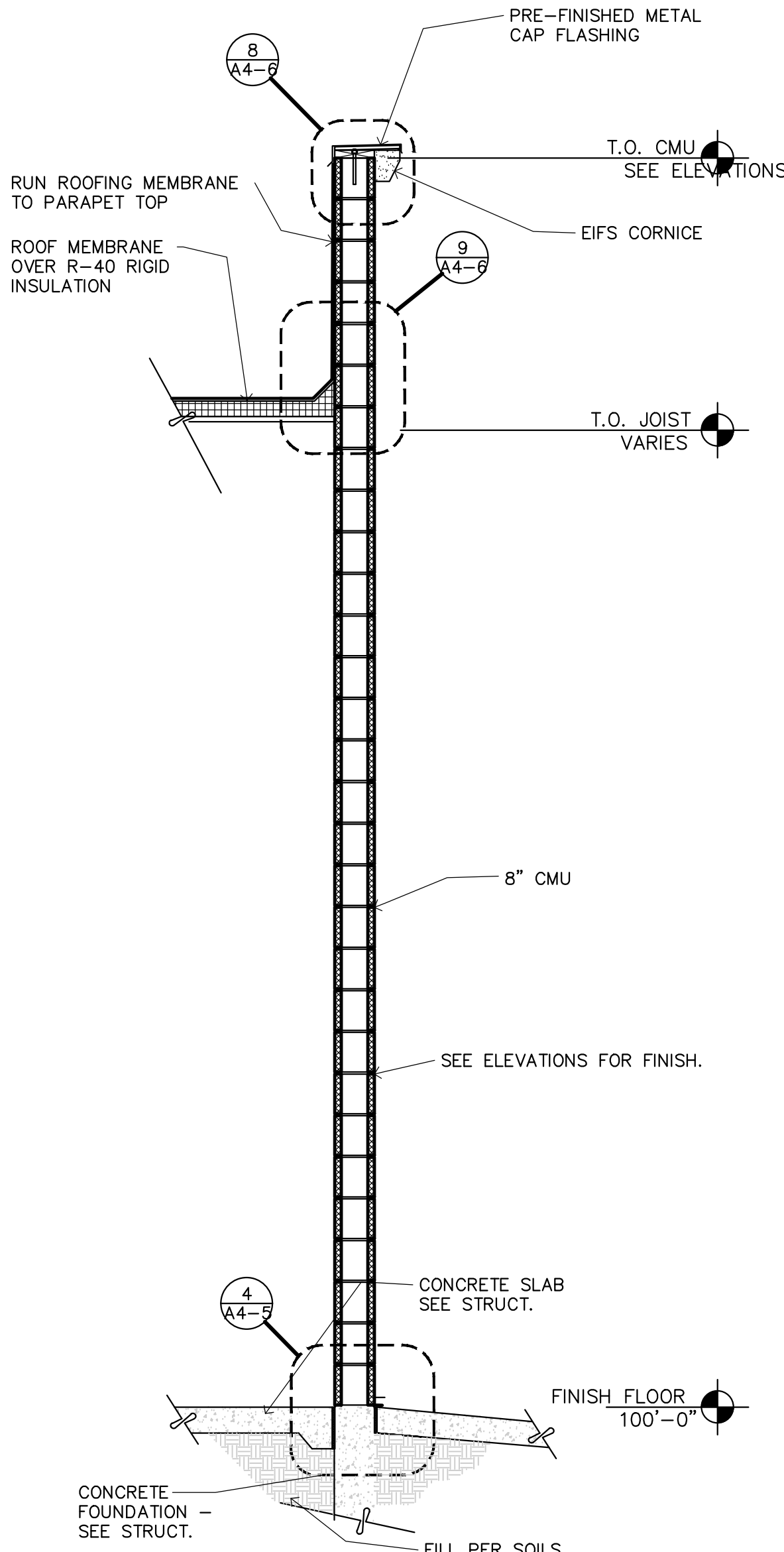
4 WALL SECTION
A4-3 1/2"=1'-0"



5 FAUX STONE WATER DRAINAGE DETAILS.
A4-3 NO SCALE



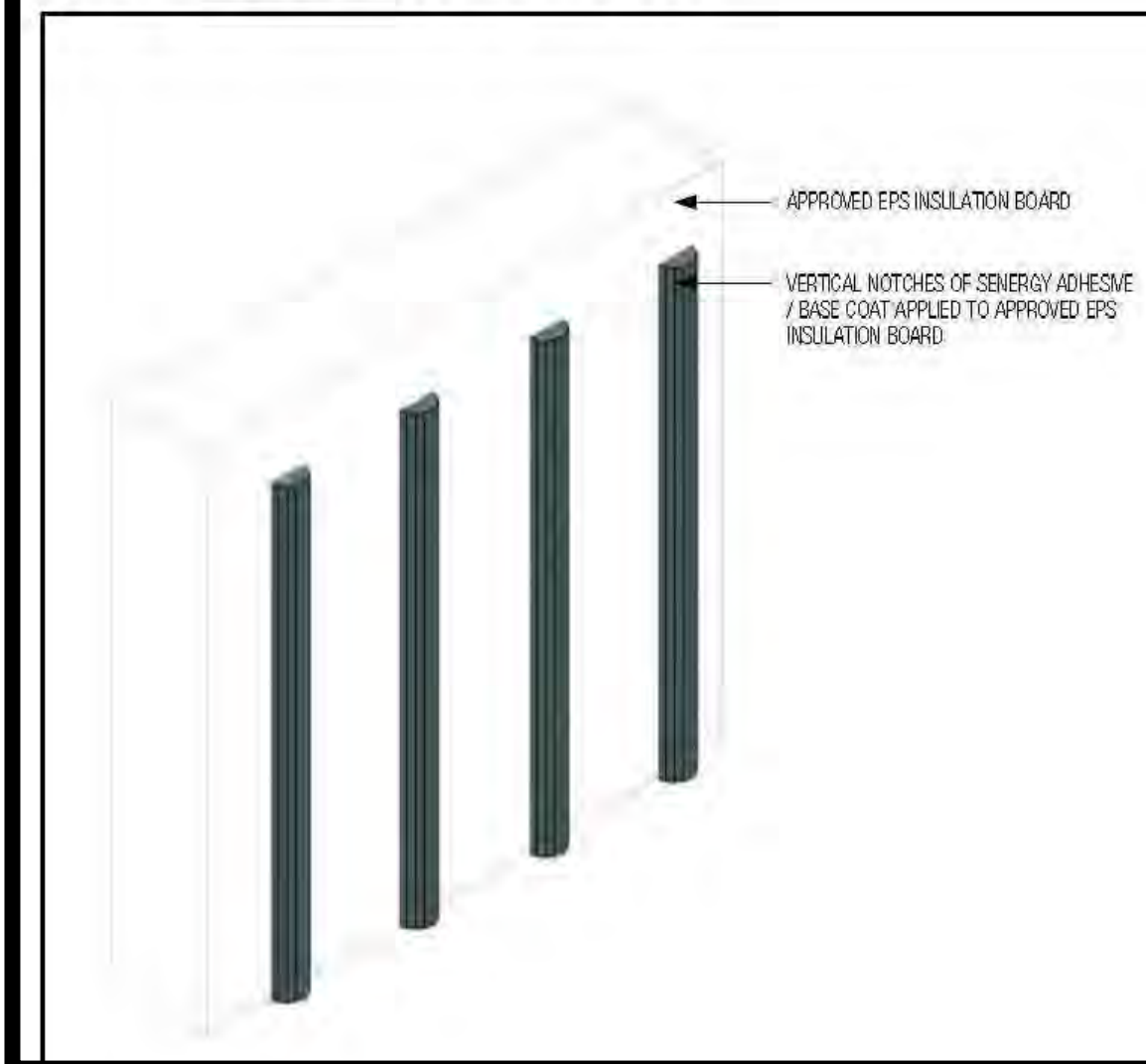
3 WALL SECTION
A4-3 1/2"=1'-0"



2 WALL SECTION
A4-3 1/2"=1'-0"

Channeled Adhesive CI Design

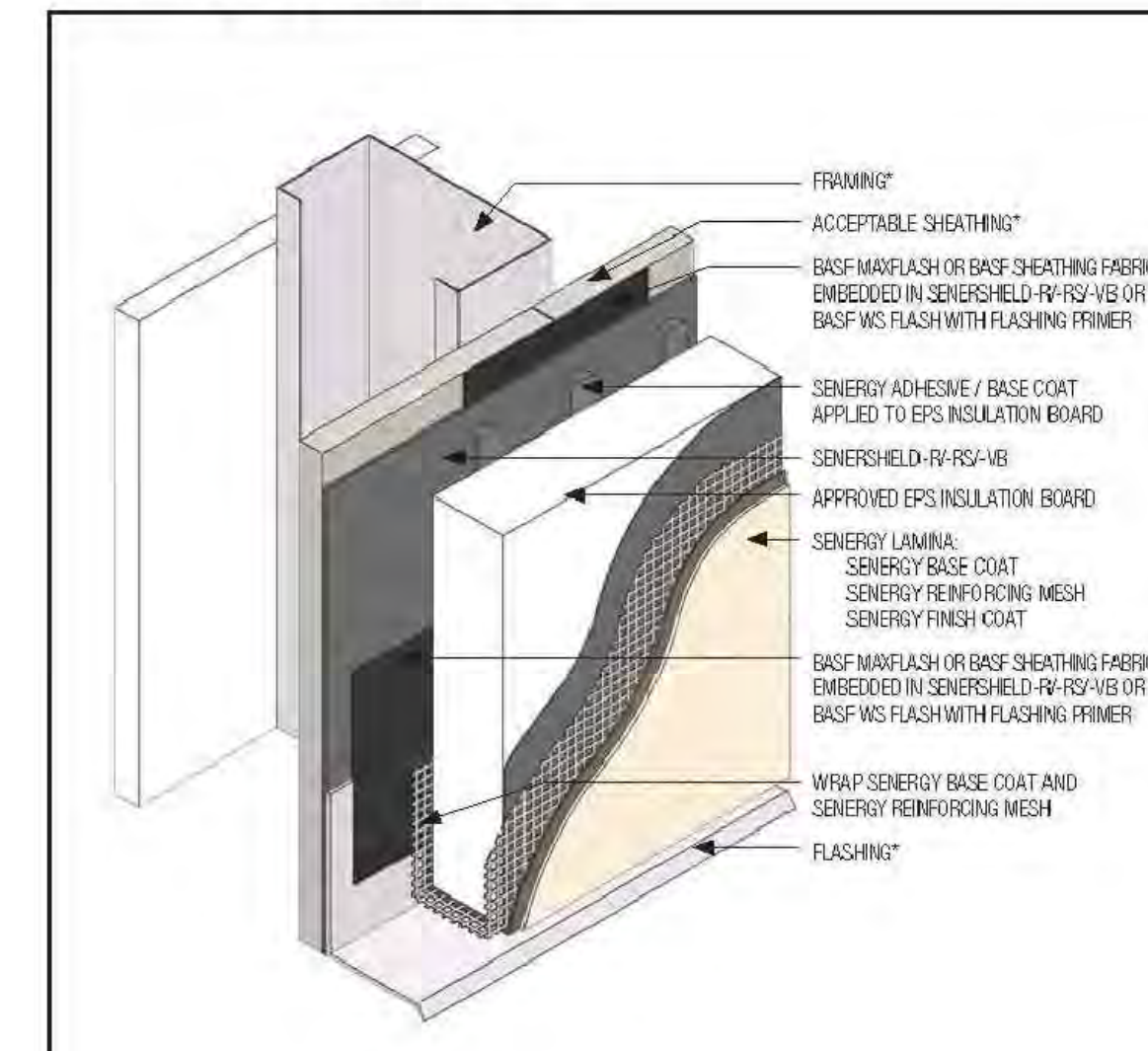
TYPICAL CHANNELLED ADHESIVE



- Install BASF materials in accordance with current installation instructions.
- Unsatisfactory conditions shall be reported to the General Contractor and corrected before the application of BASF products.
- Verify all materials are installed in accordance with current installation instructions.
- Apply mixed base coat to entire surface of insulation board using a stainless steel trowel with 1/2" x 1/2" (13 mm x 13 mm) notches spaced 2" (50 mm) apart. Ribbons of adhesive must be applied parallel to the 2' (610 mm) dimension of the EPS insulation board to ensure they are vertical when the EPS insulation board is applied to the substrate.
- Set EPS insulation board into place and apply pressure over entire surface of board to ensure positive uniform contact and high initial grab. Do not slide board into place.

Channeled Adhesive CI Design

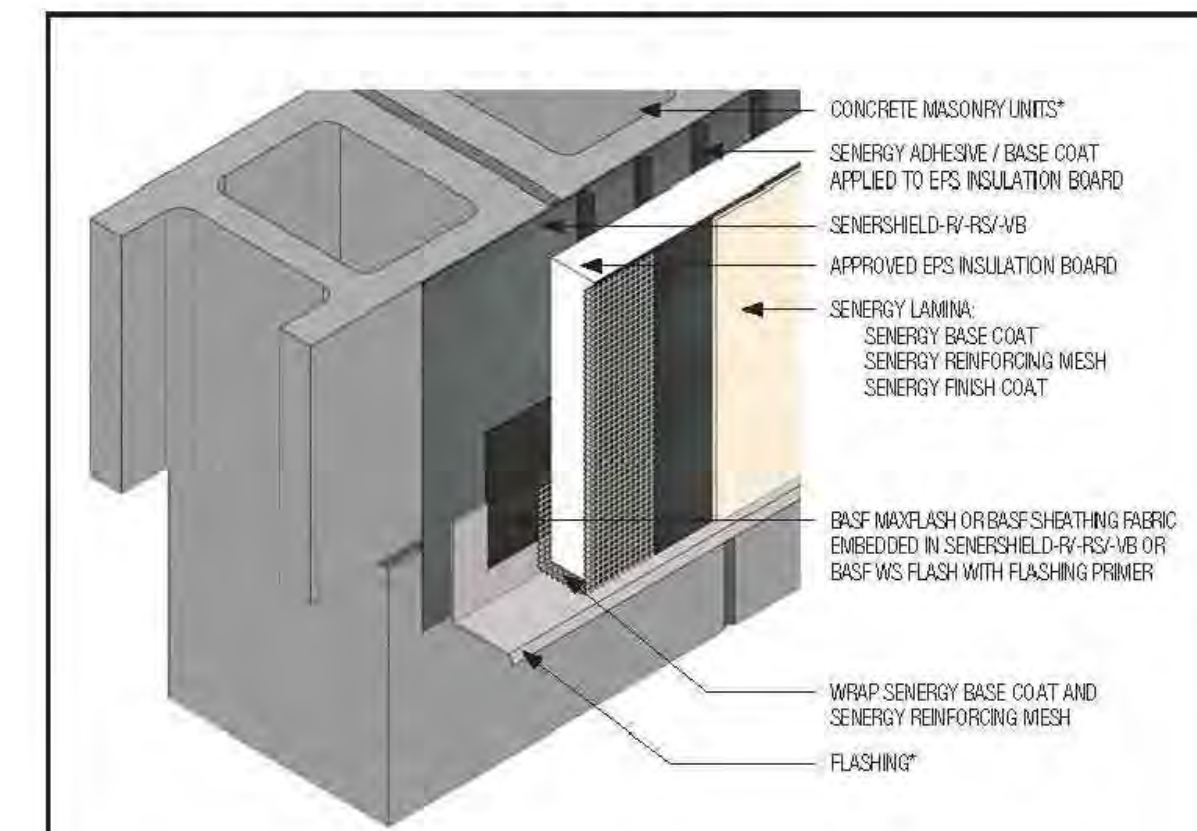
TYPICAL APPLICATION



- Install BASF materials in accordance with current installation instructions.
- Unsatisfactory conditions shall be reported to the General Contractor and corrected before the application of BASF products.
- Verify all materials are installed in accordance with current installation instructions.
- All terminations must be fully encapsulated with mesh reinforced base coat.
- Ensure a means for drainage is provided at system termination.

Channeled Adhesive CI Design

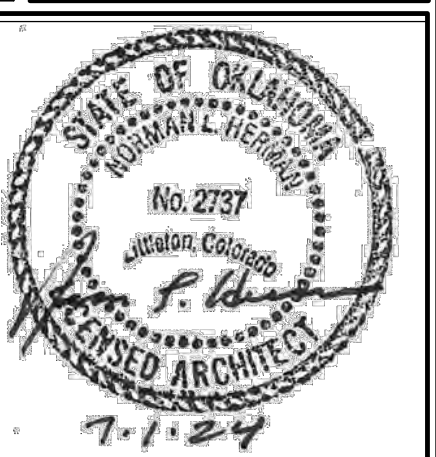
TYPICAL APPLICATION OVER CMU



- Install BASF materials in accordance with current installation instructions.
- Unsatisfactory conditions shall be reported to the General Contractor and corrected before the application of BASF products.
- Verify all materials are installed in accordance with current installation instructions.
- All terminations must be fully encapsulated with mesh reinforced base coat.
- Ensure a means for drainage is provided at system termination.

BRAKES PLUS

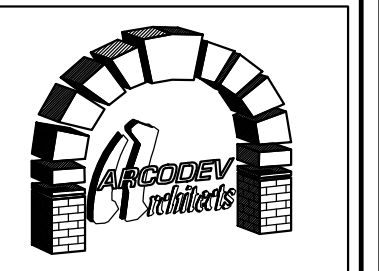
2505 MAIN STREET
NORMAN, OKLAHOMA



ARCHITECT OF RECORD

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1	07.01.24	FOR BLDG. DEPT. SUBMITTAL
	07.23.24	ADDENDUM 1

ARC00REV JOB #:
CLIENT JOB #:
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45 SPYGLASS DRIVE
LITTLETON, CO 80123
VOICE: 303.681-8925

SHEET

A4-3

WALL SECTIONS

GENERAL STRUCTURAL NOTES:

A. DESIGN DATA:

DESIGN CODE:	2018 INTERNATIONAL BUILDING CODE
CONCRETE 28-DAY STRENGTH:	FC = 4,000 PSI
MISCELLANEOUS ROLLED SECTIONS AND PLATES (ANGLES, CHANNELS, PLATES, ETC.)	ASTM A36 (UNLESS NOTED OTHERWISE)
PLAIN BOLTS AND ANCHORS	ASTM A307
ANCHOR RODS	ASTM F1554, GRADE 36 KSI
REINFORCING STEEL	ASTM A615 FY = 60,000 PSI
WELDED WIRE FABRIC	ASTM A185

MORTAR TYPE S & GROUT 28-DAY COMPRESSIVE STRENGTH = 2,000 PSI (MASONRY CEMENT IS NOT ACCEPTABLE)ASTM C1019

CONCRETE MASONRY UNITS (LIGHTWEIGHT) ASTM C652
NET COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS = 2,800 PSI
NET AREA COMPRESSIVE STRENGTH OF MASONRY FM = 2,000 PSI

ALLOWABLE SOIL BEARING CAPACITY: 2,500 PSF ON IMPROVED SUBGRADE (PER SOIL REPORT)

DESIGN LOADS
ROOFS D = 20 PSF
 Lr = 20 PSF
 S = 10 PSF (GROUND & ROOF SNOW LOAD)

WIND LOADING CRITERIA (2018 IBC & ASCE 7-16)
115 MPH, EXPOSURE C

SEISMIC LOADING CRITERIA (2018 IBC & ASCE 7-16)
IMPORTANCE FACTOR = 1.0
MAPPED SPECTRAL RESPONSE Ss = 0.346g, S1 = 0.086g
SITE CLASS = D, Fa = 1.52, Fv =2.4
SPECTRAL RESPONSE COEFFICIENTS: SDS = 0.352g, SD1 = 0.137g
SEISMIC DESIGN CATEGORY = C
SEISMIC FORCE RESISTANT SYSTEM = ORDINARY REINFORCED MASONRY SHEAR WALLS & LIGHT FRAMED SHEAR WALLS
RESPONSE MODIFICATION FACTOR (R) = 2.0

B. FOUNDATION WORK:

1. THE GEOTECHNICAL REPORT PREPARED BY OLSSON (PROJECT NO. 023-07612) DATED JANUARY 19, 2024, IS AVAILABLE AND SHALL BE REVIEWED BY THE CONTRACTOR. SEE SPECIFICATIONS AND GEOTECHNICAL REPORT FOR OVEREXCAVATION RECOMPACTION.

2. CONTRACTOR SHALL COORDINATE FOOTING ELEVATIONS WITH FINAL GRADING PLAN TO PROVIDE A MINIMUM OF 2'-0" OF GRADE ABOVE THE BOTTOM OF ALL FOOTINGS.

3. SUBSOILS SUPPORTING OR IN DIRECT CONTACT WITH FOOTINGS, SLABS/ON/GRADE, OR OTHER FOUNDATION ELEMENTS SHALL BE PROTECTED AGAINST FREEZING CONDITIONS THAT COULD CAUSE MOVEMENT OR OTHER DETRIMENTAL EFFECT TO THE STRUCTURE AS A WHOLE OR TO ANY OF ITS COMPONENT PARTS.

4. WHEN WORKING NEAR EXISTING AND/OR NEW CONSTRUCTION, THE CONTRACTOR SHALL EXERCISE EXTREME CAUTION SO AS NOT TO UNDERMINE, DISTURB, DAMAGE OR, IN ANY WAY, CAUSE UNDESIRABLE MOVEMENT, CRACKING, AND/OR SETTLEMENT OF THE ADJACENT CONSTRUCTION.

5. SLABS ON GRADE SHALL BE SUPPORTED ON SUBGRADE THAT HAS BEEN CONSTRUCTED IN ACCORDANCE WITH THE SECTION 4.3 "SLAB-ON-GRADE FLOORS" IN THE GEOTECHNICAL REPORT. ANY UNACCEPTABLE UNDISTURBED VIRGIN SOIL OR BACKFILL/GRANULAR FILL, AS DETERMINED BY THE OWNER'S GEOTECHNICAL ENGINEER, SHALL BE REMOVED AND REPLACED AS REQUIRED BY THE GEOTECHNICAL ENGINEER.

C. CONCRETE:

1. FOR REINFORCEMENT DEVELOPMENT LENGTH AND SPLICE LENGTH SEE TYPICAL REINFORCEMENT TABLE ON THIS SHEET.

2. PROVIDE CORNER BARS IN WALLS AND FOOTINGS THE SAME SIZE AND NUMBER AS THE CONTINUOUS REINFORCING.

3. REINFORCING IN FOOTINGS SHALL BE ACCURATELY PLACED BEFORE PLACING CONCRETE. DO NOT FLOAT REINFORCING INTO FOOTINGS.

4. CONCRETE SHALL BE REGULAR WEIGHT (144 PCF) WITH TYPE VII CEMENT, 3/4" MAXIMUM AGGREGATE SIZE WITH POTABLE WATER. CONCRETE SHALL CONFORM TO ACI 301. THE MAXIMUM WATER-CEMENT RATIO FOR FOOTINGS, WALLS & SLABS SHALL BE 0.45. PROVIDE 6% AIR ENTRAINMENT IN CONCRETE USED IN FOOTINGS & WALLS. INTERIOR SLABS SHALL HAVE NATURAL ENTRAPPED AIR (3% MAXIMUM).

5. MECHANICALLY VIBRATE CONCRETE, EXCEPT THAT SLABS ON GRADE NEED TO BE VIBRATED ONLY AROUND UNDERFLOOR DUCTS AND OTHER ITEMS EMBEDDED IN THE SLAB. EMBEDDED ITEMS INCLUDE ELECTRICAL CONDUITS, MECHANICAL PIPING, AND STEEL ANGLES OR CHANNELS. EMBEDDED ITEMS DOES NOT IMPLY REINFORCING STEEL. ALL OTHER CONCRETE PLACEMENT SHALL BE VIBRATED. CONCRETE SHALL BE VIBRATED IN CONFORMANCE WITH ACI 309. VIBRATE CONCRETE ONLY UNTIL THE CONCRETE IS THOROUGHLY CONSOLIDATED AND THE VOIDS FILL. INSERT INTERNAL VIBRATORS VERTICALLY TO THE FULL DEPTH OF THE LAYER BEING PLACED AND INTO THE PREVIOUS LAYER IF APPLICABLE. DO NOT DRAG VIBRATORS THROUGH THE CONCRETE. DO NOT FLOW CONCRETE FROM ONE LOCATION TO ANOTHER BY USE OF VIBRATOR.

6. DO NOT PLACE PIPES, DUCTS, OR CHASES IN STRUCTURAL CONCRETE WITHOUT APPROVAL OF THE ARCHITECT/ENGINEER. SEE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR LOCATIONS.

7. FLOOR SURFACE TOLERANCE CLASS "B". SEE ACI 301 FOR PROCEDURE OF MEASUREMENT AND CORRECTION.

8. CONTROL JOINTS SHALL BE PLACED AT COLUMN/LINE INTERSECTIONS AT A MAXIMUM SPACING INDICATED BELOW AND HAVE A MAXIMUM ASPECT RATIO OF 1.5 TO 1.0 UNLESS OTHERWISE INDICATED. SEE DETAIL - ON DRAWING - FOR CONTROL JOINT REQUIREMENTS.

SLAB THICKNESS	MAX. CONTROL JOINT SPACING
4"	12'-0"
5"	15'-0"

9. ALL CONSTRUCTION JOINTS IN CONCRETE WALLS SHALL HAVE A 2" X 4" CONTINUOUS KEYWAY. ALL CONSTRUCTION JOINTS, EXCEPT THOSE DETAILED, SHALL HAVE ARCHITECT/ENGINEER APPROVAL. SEE SPECIFICATIONS FOR OTHER CONSTRUCTION JOINT REQUIREMENTS.

10. ALL REINFORCING STEEL SHALL BE DEFORMED NEW BILLETS BARS (A615, GRADE 60), BENT COLD, AND DETAILED, FABRICATED, AND HELD IN PLACE IN ACCORDANCE WITH THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" (ACI 315 - LATEST EDITION) EXCEPT AS OTHERWISE DETAILED OR SPECIFIED.

11. UNLESS NOTED OTHERWISE ON PLAN SHEETS SLABS ON GRADE SHALL BE:

SLAB THICKNESS	SLAB REINFORCEMENT	SUBBASE
4"	6x6-W1 4W1.4 WWF	SEE GEOTECHNICAL REPORT
5"	6x6-W2.3xW2.3 WWF	SEE GEOTECHNICAL REPORT

12. ALL REINFORCING IN SLABS AND WALLS SHALL BE CONTINUOUS UNLESS DETAILED OTHERWISE AND LAP SPLICED ONLY IN REGIONS OF LOW STRESS. ALL BARS SHALL HAVE A STANDARD HOOK WHERE A HOOK IS SHOWN, UNLESS DETAILED OTHERWISE.

13. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING, UNLESS NOTED OTHERWISE:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
CONCRETE EXPOSED TO EARTH OR WEATHER: 2"
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS AND WALLS: 1"

D. MASONRY:

1. FURNISH AND CONSTRUCT MASONRY IN ACCORDANCE WITH THE REQUIREMENTS OF THE SPECIFICATIONS FOR MASONRY CONSTRUCTION (ACI 530.1-11/ASCE 6-11/TMS 602-11.)

2. LAY MASONRY UNITS IN RUNNING BOND.

3. MAXIMUM GROUT LIFT WITHOUT CLEANOUTS 4'-0" IN BLOCK WALLS AND 8" IN GROUTED TWO-WYTHE WALLS.

4. IN 8" WALLS, PROVIDE CONTINUOUS FULL HEIGHT VERTICAL REINFORCING IN CENTER OF GROUT AT CENTER OF WALL. TYPICAL REINFORCING SHALL BE 1-#6 AT 2'-8" ON CENTER AND 2-#6 AT CORNERS, INTERSECTIONS, WALL ENDS, DOOR AND WINDOW JAMBS, AND SIDE OF EXPANSION OR CONTROL JOINTS UNLESS NOTED OTHERWISE.

5. GROUT CELLS FULL AT ALL ANCHOR AND EMBED LOCATIONS.

6. PROVIDE LADDER TYPE #9 JOINT REINFORCING AT 16" ON CENTER VERTICAL SPACING IN ALL CLAY MASONRY AND UNLESS NOTED OTHERWISE.

7. SPLICE MASONRY WALL REINFORCING AS SCHEDULED ON 3/S3-3.

8. PLACE BOND BEAM REINFORCING CONTINUOUS THROUGH EXPANSION/ CONTROL JOINTS, WRAPPING BARS WITH 18 INCH THICK BOND BEARING TAPE 2'-0" BOTH SIDES OF JOINT. DO NOT SPLICE BOND BEAM REINFORCING WITHIN 6'-0" OF AN EXPANSION OR CONTROL JOINT.

9. PROVIDE CONTINUOUS BOND BEAMS AT ALL BEAM BEARING AND TRUSS BEARING ELEVATIONS, AND AT THE TOP OF ALL WALLS.

10. SPLICE BOND BEAM REINFORCING AT MASONRY CONTROL/EXPANSION JOINTS AS SHOWN ON MASONRY JOINT DETAIL ON 4/S3-3.

11. PROVIDE CONTINUOUS WIRE LATH GROUT BARRIERS BELOW BOND BEAMS.

12. PROVIDE LINTELS OVER ALL OPENINGS AND RECESSES IN MASONRY WALLS.

13. ALL LINTELS SHALL HAVE A MINIMUM BEARING OF 8 INCHES EACH END.

14. FOR ALL OPENINGS NOT OTHERWISE DETAILED OR SCHEDULED, MINIMUM LINTELS SHALL BE FOR EACH 4 INCH OF MASONRY WIDTH 1-L 3-1/2 X 3-1/2 X 1/4 FOR SPANS UP TO 4'-0", 1-L 4 X 3-1/2 X 1/4 FOR SPANS UP TO 6'-0" AND 1-L 5 X 3-1/2 X 1/4 FOR SPANS UP TO 8'-0". FOR SPANS LESS THAN 2'-0" PROVIDE A 5/16" PLATE.

15. MASONRY CONTROL JOINT SPACING SHALL NOT EXCEED 24'-0".

16. ALL MASONRY CONSTRUCTION SHALL HAVE SPECIAL INSPECTION PER IBC SECTION 1705.4 AND HAVE LEVEL 2 QUALITY ASSURANCE IN ACCORD WITH ACI530-11 SECTION 1.14. PERIODIC INSPECTION SHALL BE INTERPRETED AS TWO TIMES PER WEEK.

E. WOOD:

1. WOOD SHALL COMPLY WITH THE LATEST EDITION OF THE GRADING RULES OF THE WESTERN WOOD PRODUCTS ASSOCIATION OR THE WEST COAST LUMBER INSPECTION BUREAU.

2. ALL SAWN LUMBER SHALL BE STAMPED WITH THE GRADE MARK OF AN APPROVED GRADING AGENCY.

3. SAWN LUMBER SHALL HAVE THE FOLLOWING MINIMUM GRADE UNLESS NOTED OTHERWISE:

TYPE OF USE	MATERIAL AND GRADE
TOP PLATES, ALL OTHER SAWN LUMBER	DOUGLAS FIR NO. 2
POSTS AND BEAMS	DOUGLAS FIR NO. 1

4. ALL 2x BEARING WALLS SHALL BE BLOCKED HORIZONTALLY AT 4'-0" O.C. VERT. SPACING FOR ALL WALLS GREATER THAN 9'-0" IN HEIGHT.

5. ALL PLYWOOD SHALL BE C-D OR C-C SHEATHING EXTERIOR GRADE CONFORMING TO STANDARD PS0.019.

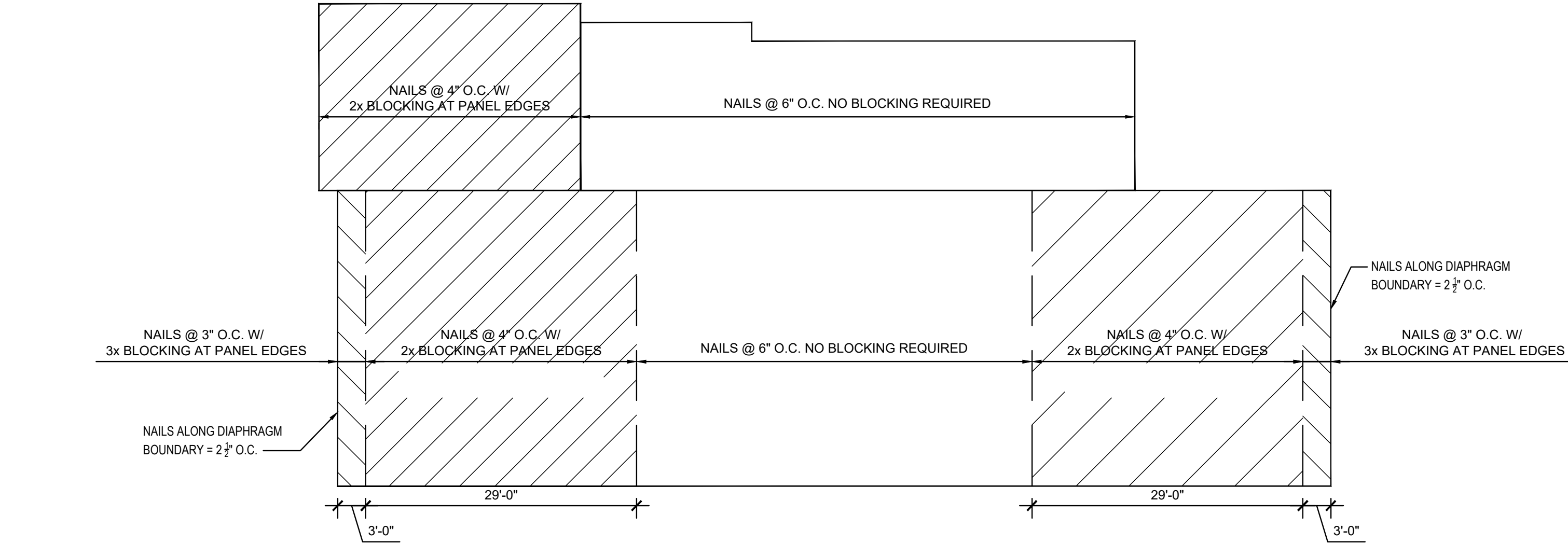
6. PLYWOOD SHALL BE PLACED WITH FACE GRAIN PERPENDICULAR TO SUPPORTS USING A MINIMUM 5-PLY PLYWOOD. PLYWOOD JOINTS SHALL BE STAGGERED.

7. PLYWOOD ATTACHMENT SHALL BE DONE USING COMMON NAILS. NAILING SHALL BE AS NOTED ON ROOF FRAMING PLAN.

8. ALL SILL PLATES IN CONTACT WITH CONCRETE OR MASONRY SHALL BE TREATED DOUGLAS FIR OR FOUNDATION GRADE REDWOOD.

9. BRACE WOOD TRUSSES Laterally AT BEARING POINTS AND INTERMEDIATE LOCATIONS AS REQUIRED BY MANUFACTURER.

10. SEE 3/S1-0 FOR ULTIMATE WIND UPLIFT.



2 S1-0 3/4"=1'-0" ROOF DIAPHRAGM NAILING PATTERN

F. SPECIAL INSPECTION

1. IN ACCORD WITH 2018 IBC SECTIONS 1704 & 1705, AS NOTED BELOW, TESTING AND INSPECTION SHALL BE BY AN INDEPENDENT TESTING/INSPECTION FIRM UNDER THE SUPERVISION OF A LICENSED ENGINEER EMPLOYED BY THAT FIRM. THIS ENGINEER SHALL BE DEEMED THE DESIGNATED ENGINEER OF RECORD FOR SPECIAL INSPECTIONS PERFORMED BY HIS FIRM OR HIS CONSULTANTS. INSPECTORS SHALL BE ICBO CERTIFIED AND APPROVED BY THE BUILDING OFFICIAL.

2. THE DESIGNATED ENGINEER OF RECORD FOR SPECIAL INSPECTIONS SHALL BE RESPONSIBLE FOR DEFINING THE ACTIVITIES OF THE INSPECTORS, FOR CERTIFYING THE QUALIFICATIONS OF THE INSPECTORS WITH THE BUILDING OFFICIAL AND TO ATTEND THE PRE-CONSTRUCTION MEETING TO DEFINE THEIR SCOPE OF SERVICES AND THE TESTING OR TEST PROCEDURES THAT ARE REQUIRED AS OUTLINED IN THE INTERNATIONAL BUILDING CODE.

3. SPECIAL INSPECTION IS TO BE PROVIDED IN ADDITION THE INSPECTIONS CONDUCTED BY THE LOCAL DEPARTMENT OF BUILDING SAFETY AND SHALL NOT BE CONSTRUED TO RELIEVE THE OWNER OR HIS AUTHORIZED AGENT FROM REQUESTING THE PERIODIC AND CALLED INSPECTIONS REQUIRED BY SECTION 108.5 AND 108.7 OF THE INTERNATIONAL BUILDING CODE.

4. CONCRETE PER SECTION 1705.3 AND TABLE 1705.3.

5. ANCHOR RODS INSTALLED IN CONCRETE; PER TABLE 1705.3.

6. REINFORCING PER TABLE 1705.3.

7. STRUCTURAL MASONRY; PER SECTION 1705.4.

8. GRADING, EXCAVATION AND FILLING; PER SECTION 1705.6. SEE GEOTECHNICAL REPORT.

9. EXPANSION BOLT, SCREW ANCHOR AND ADHESIVE ANCHOR INSTALLATION TO VERIFY INSTALLATION IN ACCORD WITH ICBO REPORTS NOTED PREVIOUSLY OR APPROVED EQUAL.

10. THE INSPECTOR SHALL OBSERVE THE WORK ASSIGNED TO BE CERTAIN IT CONFORMS TO THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS.

11. THE INSPECTOR SHALL FURNISH DAILY INSPECTION REPORTS ON THE WORK TO THE BUILDING OFFICIAL AND TO THE ENGINEER. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, AND, IF UNCORRECTED, TO THE ENGINEER AND THE BUILDING OFFICIAL.

12. THE TESTING/ INSPECTION FIRMS ENGINEER SHALL COMPLETE, SIGN AND SEAL, A FINAL REPORT CERTIFYING THAT TO THE BEST OF HIS KNOWLEDGE, THE WORK IS IN CONFORMANCE WITH THE CONTRACT DOCUMENTS.

13. THE SPECIAL INSPECTOR SHALL BE SELECTED AND CREDENTIALS SHALL BE SUBMITTED TO THE CITY PRIOR TO THE ISSUANCE OF A BUILDING PERMIT.

G. OTHER:

1. UNLESS NOTED OTHERWISE, EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT II EXPANSION ANCHORS OR APPROVED EQUAL. ADHESIVE ANCHORS SHALL BE HILTI STANDARD HAS RODS WITH THE HVA ADHESIVE SYSTEM, THE SIMPSON SET SYSTEM, OR APPROVED EQUAL.

2. VERIFY ALL DIMENSIONS AND CONDITIONS PRIOR TO STARTING WORK. NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES OR INCONSISTENCIES.

3. VERIFY IN FIELD ALL EXISTING CONDITIONS SHOWN ON DRAWINGS.

4. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR MECHANICAL, ELECTRICAL, AND PLUMBING WITH APPROPRIATE TRADES. PROVIDE ALL TEMPORARY BRACING, SHORING, GUYING, OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION.

5. ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE STAMP (AND SIGNATURE) OF AN ENGINEER REGISTERED IN OKLAHOMA.

TYPICAL REINFORCING NOTES

1. REINFORCING BAR DEVELOPMENT AND LAP SPODE LENGTH SHALL BE AS SHOWN IN THIS TABLES UNLESS OTHERWISE NOTED ON THE DRAWINGS.

2. THE LENGTHS SHOWN IN THE TABLES ARE BASED ON THE FOLLOWING CONCRETE COVERAGE AND REINFORCING C-C SPACING:

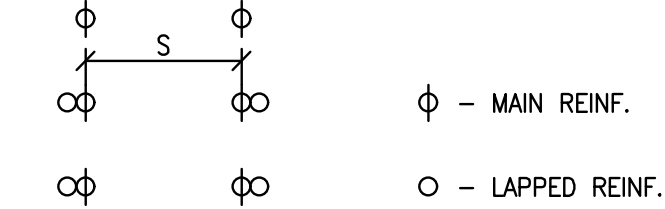
BEAMS OR COLUMNS:
COVER (EQUAL OR MORE) 1.0bd (BAR DIAMETER)
CENTER TO CENTER (C-C) SPACING (EQUAL OR MORE) 2.0bd.
ALL OTHERS:
COVER (EQUAL OR MORE) 1.0bd
CENTER TO CENTER SPACING (EQUAL OR MORE) 3.0bd.

3. TOP BARS ARE DEFINED AS HORIZONTAL REINFORCEMENT SUCH THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPLICE.

4. DEVELOPMENT AND SPLICE LENGTH SHOWN SHALL NOT APPLY IF ANY OF THE FOLLOWING CONDITIONS OCCUR:

A) fc < 2,500 PSI
B) fy > 60,000 PSI
C) THE COVER OR C-C BAR SPACING IS NOT AS LISTED ABOVE
D) THE REINFORCING STEEL IS EPOXY COATED
E) LIGHT WEIGHT CONCRETE IS USED.

5. CENTER ON CENTER SPACING (S) IS DEFINED AS BELOW:



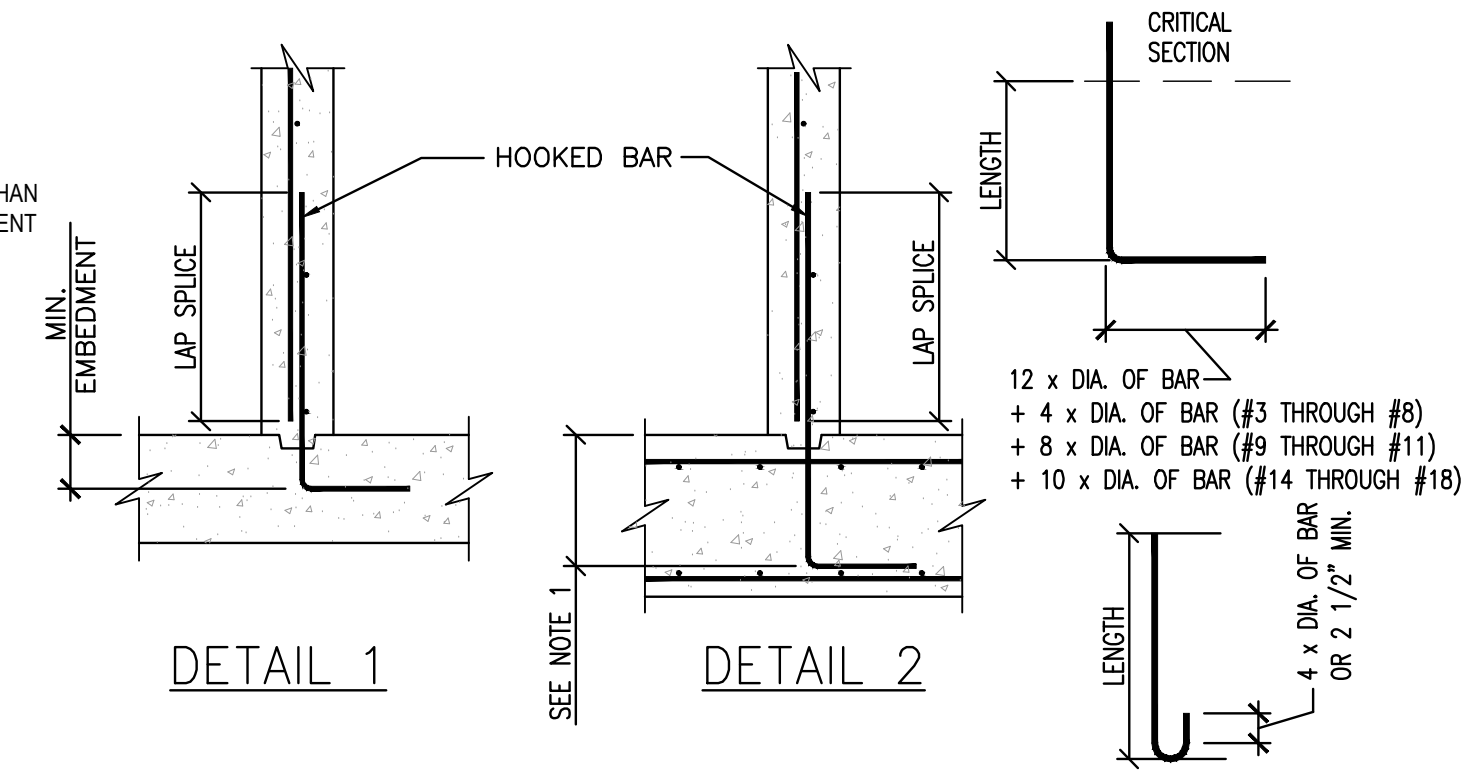
REINFORCING DEVELOPMENT AND SPLICES fc = 4,000 PSI				
BAR SIZE	DEVELOPMENT LENGTH		SPLICE LENGTH	
	TOP	OTHER	TOP	OTHER
#3	1'-7"	1'-3"	2'-0"	1'-7"
#4	2'-1"	1'-7"	2'-8"	2'-1"
#5	2'-7"	2'-0"	3'-4"	2'-7"
#6	3'-1"	2'-5"	4'-0"	3'-1"
#7	4'-6"	3'-6"	5'-10"	4'-6"
#8	5'-2"	4'-0"	6'-8"	5'-2"
#9	5'-10"	4'-6"	7'-7"	5'-10"
#10	6'-7"	5'-1"	8'-6"	6'-7"
#11	7'-3"	5'-7"	9'-5"	7'-3"

DEVELOPMENT LENGTHS HOOKED BARS (fc = 4,000 PSI)	
BAR SIZE	LENGTH OR MIN. EMBEDMENT
#3	8"
#4	10"
#5	1'-0"
#6	1'-3"
#7	1'-5"
#8	1'-7"
#9	1'-10"
#10	2'-0"
#11	2'-3"

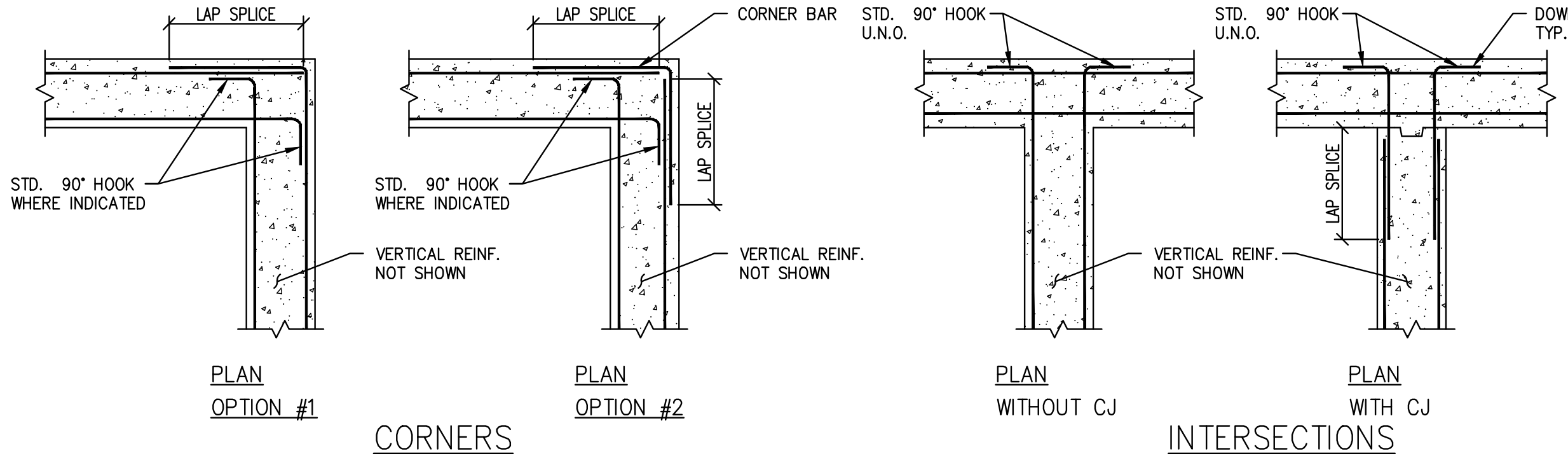
DEVELOPMENT LENGTH NOTES

1. WHERE DRAWINGS ARE DETAILED SIMILAR TO DETAIL 2, EXTEND THE EMBEDMENT LENGTH SUCH THAT THE HOOKED BAR CONTACTS THE LAYER OF MAIN REINFORCING SHOWN.

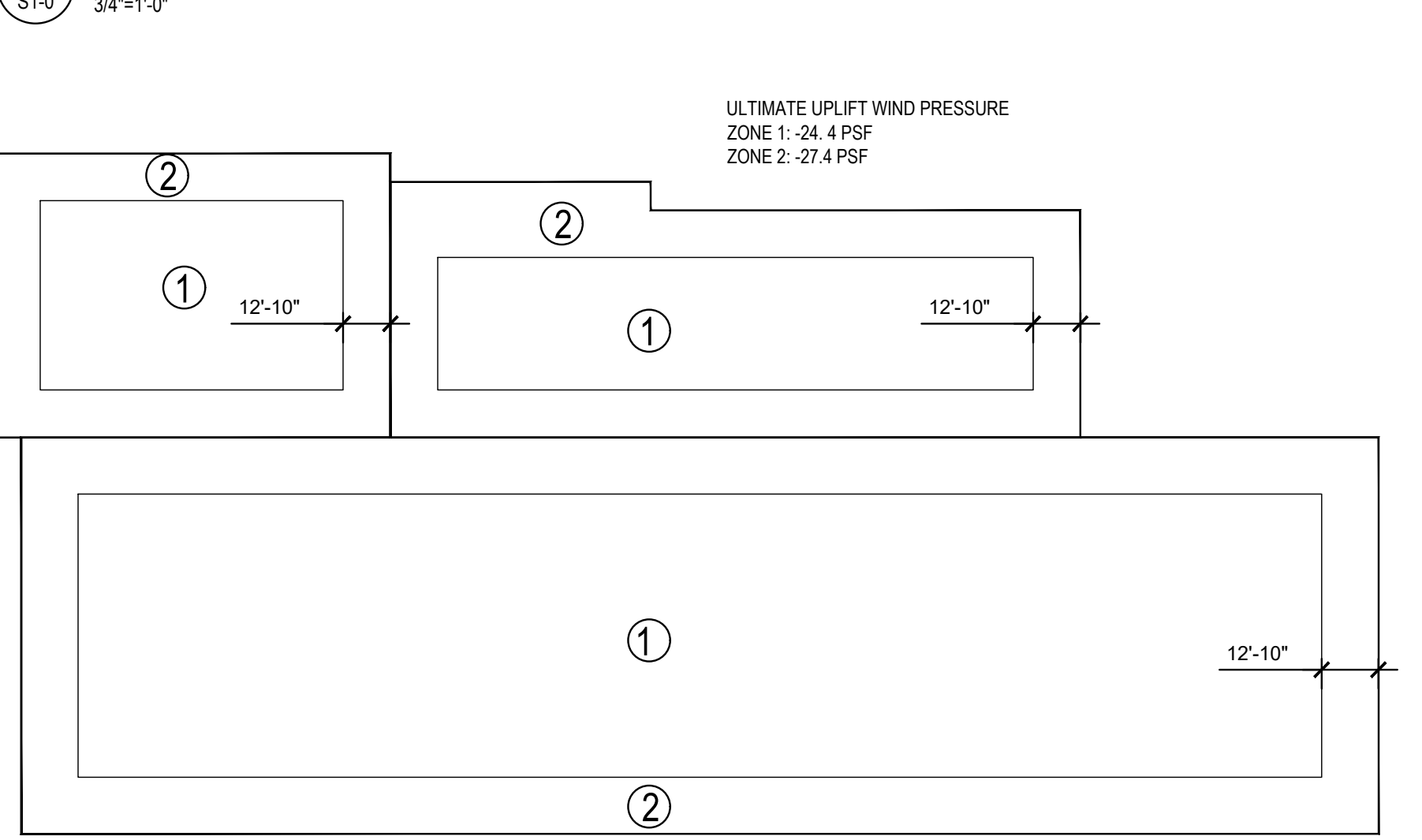
2. EMBEDMENT LENGTHS IN CHART ARE TYPICAL EXCEPT AS NOTED IN DETAIL 2, OR AS INDICATED ON DRAWINGS.



CONCRETE REINFORCEMENT COVER			
CONCRETE EXPOSURE	MEMBER	REINFORCEMENT	SPECIFIED COVER
CAST AGAINST AND PERMANENTLY IN CONTACT WITH GROUND	ALL	ALL	3"
EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	ALL	#6 TO #18 #5 AND SMALLER	2" 1 1/2"
NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	SLABS, JOISTS, & WALLS BEAMS, COLUMNS, PEDASTALS, AND TENSION TIES	#14 & #18 #14 & SMALLER PRIMARY REINFORCEMENT STIRRUPS, TIES, SPIRALS, AND HOOPS	1 1/2" 3/4" 1 1/2"



1 S1-0 3/4"=1'-0" HORIZONTAL WALL REINFORCEMENT DETAILS



3 S1-0 3/4"=1'-0" ROOF WIND UPLIFT LOADING

PERFORMANCE Engineering
11811 Fort Street, Suite 104 - Omaha, NE 68164
(402) 343-5860 Fax (402) 343-5861
NE 064605
389 Perry St., Suite 204A - Castle Rock, CO 80104
(303) 771-5322
PE # 240447

BRAKES PLUS
2505 MAIN STREET
NORMAN, OKLAHOMA

Robert A. Thorne
LICENSED PROFESSIONAL ENGINEER
OKLAHOMA
7/26/2024

ENGINEER OF RECORD

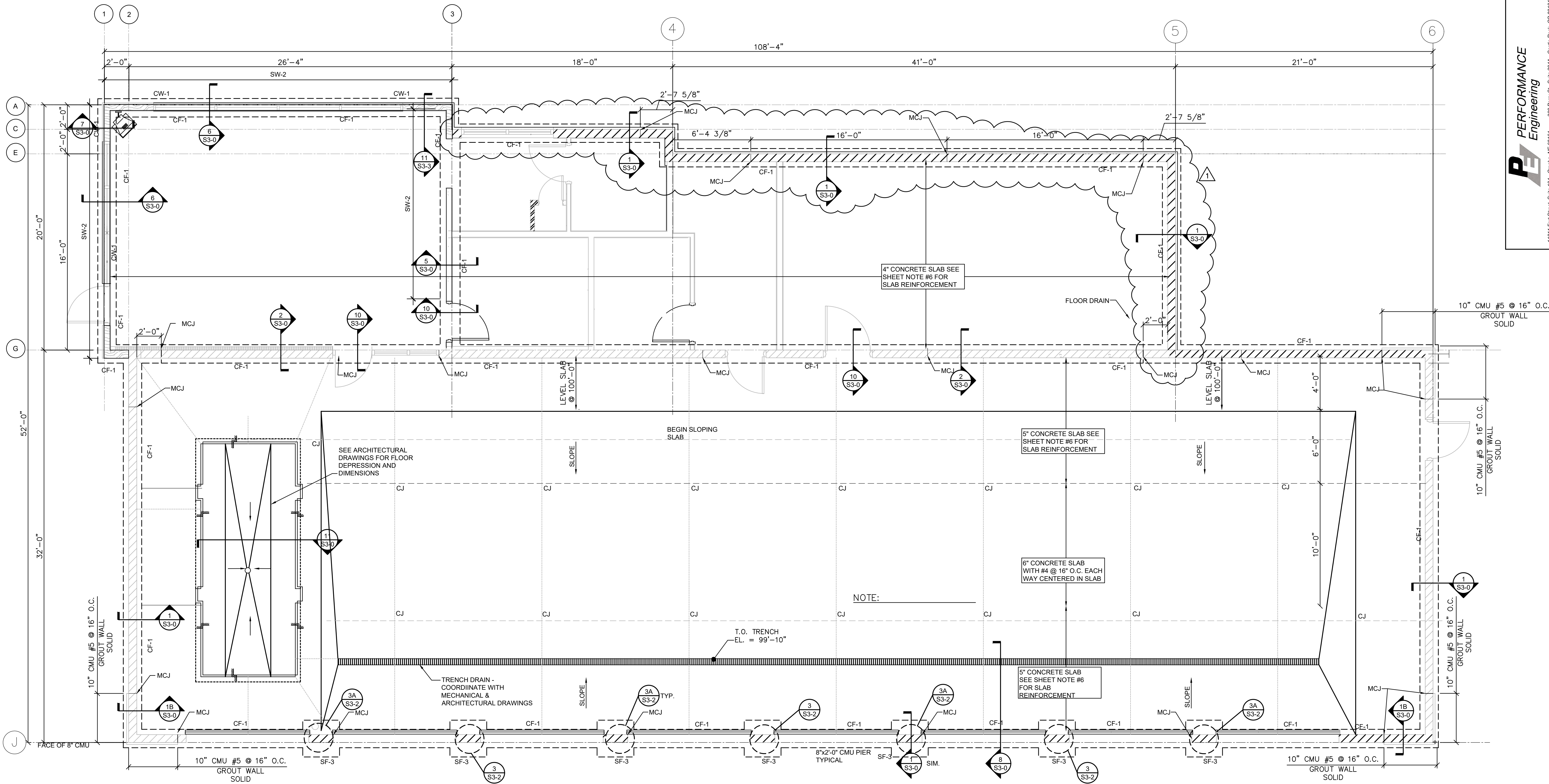
REVISION	DATE	COMMENTS
1	07.01.24	FOR BLDG. DEPT. SUBMITTAL
	07.23.24	ADDENDUM 1

ARCOWE JOB #:
CLIENT JOB #:
DRAWN BY: SLW
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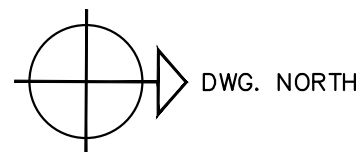
Alamy
45 SPYGLASS DRIVE
LITTLETON, CO 80123
VOICE: 303.881-8925

SHEET

S1-0
GENERAL STRUCTURAL
NOTES AND DETAILS



1 FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

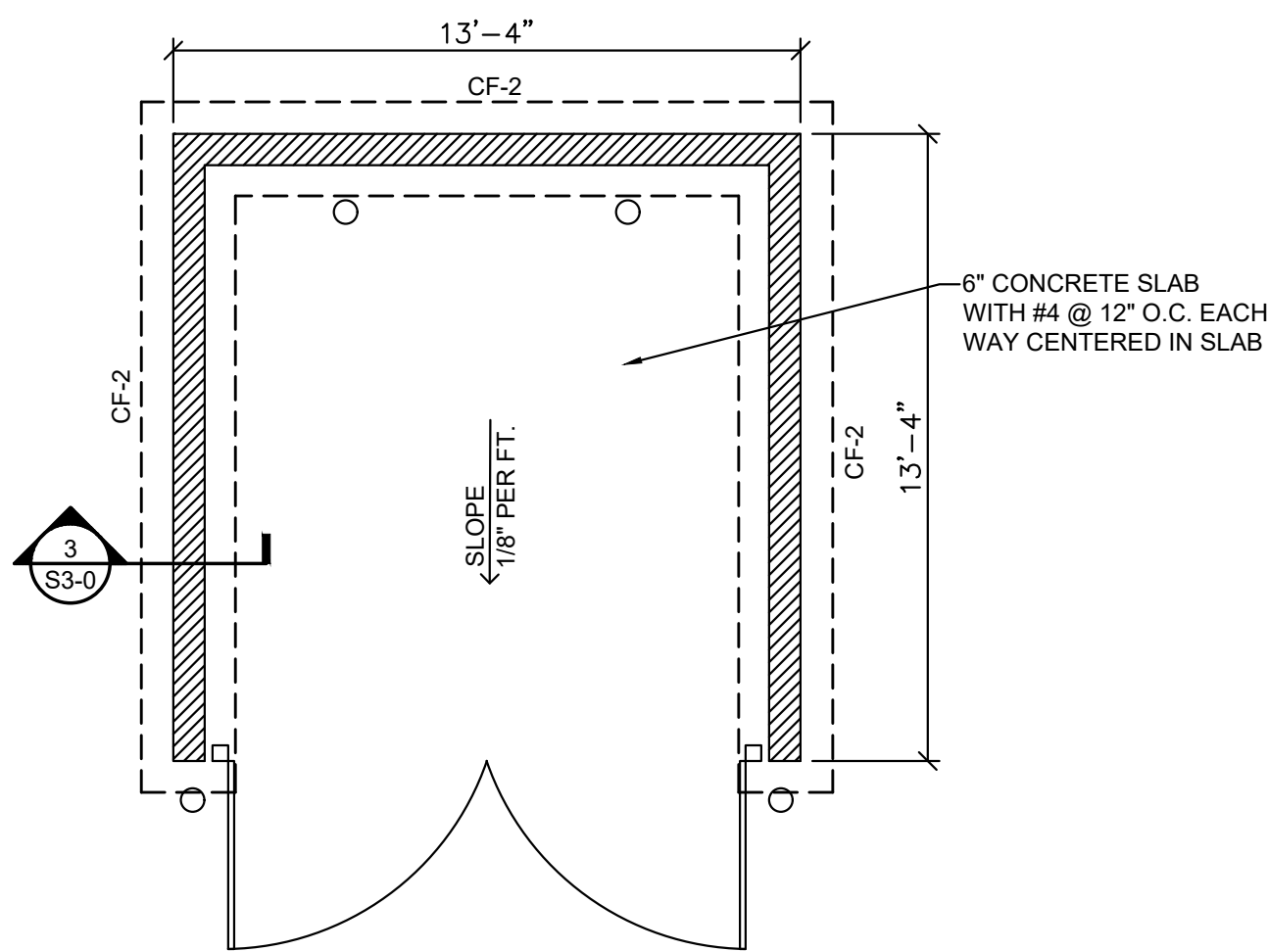


SHEAR WALL SCHEDULE									
MARK	WALL SHEATHING	NAILS, PENETRATION	PANEL EDGE NAILING	FIELD NAILING	SILL BOLTS	STUDS, SILLS & PLATES	ALLOWABLE SHEAR (PLF)	HOLDOWNS	FOUNDATION ANCHOR
SW-1	1 9/16" NOMINAL STRUCTURAL I SHEATHING, ONE SIDE, BLOCKED	10d x 3" 1 1/2" PEN.	6" O.C.	6" O.C.	3/4" DIA. @ 24" O.C.	2x STUDS, 2x SILL, 2x PL	340	HDU8-SDS2.5	7/8" DIA. EMBED 9"
SW-2	1 9/16" NOMINAL STRUCTURAL I SHEATHING, TWO SIDES, BLOCKED	10d x 3" 1 1/2" PEN.	6" O.C.	6" O.C.	3/4" DIA. @ 24" O.C.	2x STUDS, 2x SILL, 2x PL	680	HDU11-SDS2.5	1" DIA. EMBED 12"

- NOTE:
- ALL WOOD MEMBERS IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED.
 - NAILING & HARDWARE IN CONTACT WITH PRESSURE TREATED LUMBER SHALL BE HOT DIPPED GALVANIZED.
 - WHERE PANELS ARE APPLIED ON BOTH FACES OF WALL AND NAIL SPACING IS LESS THAN 6" O.C. ON EITHER SIDE PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS. STAGGER NAILING AT EACH SIDE OF JOINT.
 - INSTALL HARDWARE IN ACCORD WITH MANUFACTURER'S RECOMMENDATIONS.
 - ALL BOLT HOLES SHALL BE 1/16" (MAX) OVERSIZED AT THE CONNECTION OF HOLD DOWNS TO POSTS. INSPECTOR SHALL VERIFY.
 - SEE DETAIL 5/S3-3 FOR TYPICAL SHEAR WALL FRAMING.

FOUNDATION SCHEDULE			
MARK	SIZE (L x W x D)	REINFORCING	
CF-1	CONT. x 1'-6"x 1'-4"	3 - #5 CONT. BOTT W/ #5 TRANSVERSE @ 12" O.C.	
CF-2	CONT. x 2'-0"x 1'-4"	3 - #5 CONT. BOTT W/ #5 TRANSVERSE @ 12" O.C.	
SF-3	3'-0"x 3'-0"x 1'-4"	3 - #5 EACH WAY	

FOUNDATION WALL SCHEDULE		
MARK	SIZE	REINFORCING
CW-1	6" WIDE x 1'-2" HIGH	2 - #5 CONT. & #4 DOWELS AT 24" O.C.



2 TRASH ENCLOSURE
SCALE: 1/4" = 1'-0"

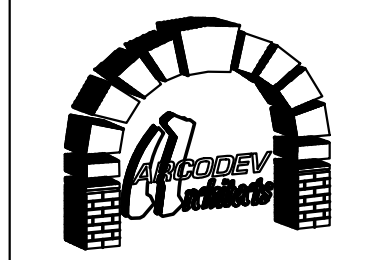
- SHEET NOTES:**
- INDICATES CMU WALLS WITH #5 VERTICAL BARS @ 2'-0" O.C. CENTERED IN MASONRY WALLS. SEE 6/S3-3 FOR MASONRY CONSTRUCTION.
 - SOIL PREPARATION BENEATH BUILDING AND FOUNDATIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE RECOMMENDATIONS SET FORTH IN THE GEOTECHNICAL REPORT FOR A MONOLITHIC REINFORCED STIFFENED SLAB ON GRADE CONSTRUCTION (OPTION #2 & TABLE 3.2 OF THE SOIL REPORT). THIS ACTION SHALL BE OBSERVED BY A REPRESENTATIVE OF THE GEOTECHNICAL FIRM ON A CONTINUOUS BASIS TO ENSURE THAT SOIL PROPERTIES MEET THE REQUIREMENTS IN THE GEOTECHNICAL REPORT.
ALL SUBGRADE SOILS BELOW SLAB ON GRADES SHALL BE MOISTURE CONDITIONED AND COMPACTED AS DISCUSSED IN THE SOIL REPORT.
CONTRACTOR SHALL FOLLOW ALL RECOMMENDATIONS FOR MATERIAL AND COMPACTION LISTED IN THE SOIL REPORT
 - CONTRACTOR SHALL COORDINATE SIZE AND LOCATION OF FLOOR AND WALL PENETRATIONS AND BLOCK OUT ACCORDINGLY.
 - TOP OF FOOTING ELEVATION SHALL BE 99'-4" UNLESS NOTED OTHERWISE. CONTINUOUS FOOTINGS MAY BE EARTH FORMED. SLABS THICKEN/HAUNCH TO 8" OVER FOOTINGS.
 - "MCJ" INDICATES MASONRY CONTROL JOINT LOCATION FOR CONCRETE MASONRY UNITS. SEE 2/S3-3 & 4/S3-3 FOR DETAILS. SEE ARCHITECTURAL DRAWINGS FOR BRICK CONTROL JOINTS.
 - SEE PLAN FOR SHOP AREA SLAB ON GRADE THICKNESS. REINFORCE CONCRETE SLAB WITH #4 BARS @ 16" O.C. EA. WAY. TOP OF SLAB SHALL BE 100'-0" UNLESS NOTED OTHERWISE.
 - SW-# INDICATES SHEAR WALL TYPE. SEE SCHEDULE.



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1	07.01.24	FOR BLDG. DEPT. SUBMITTAL
	07.23.24	ADDENDUM 1

ARC0DEV JOB # _____
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DRAWN BY: SLW
CHECKED BY: TAS
DATE OF ISSUE: 05.13.24

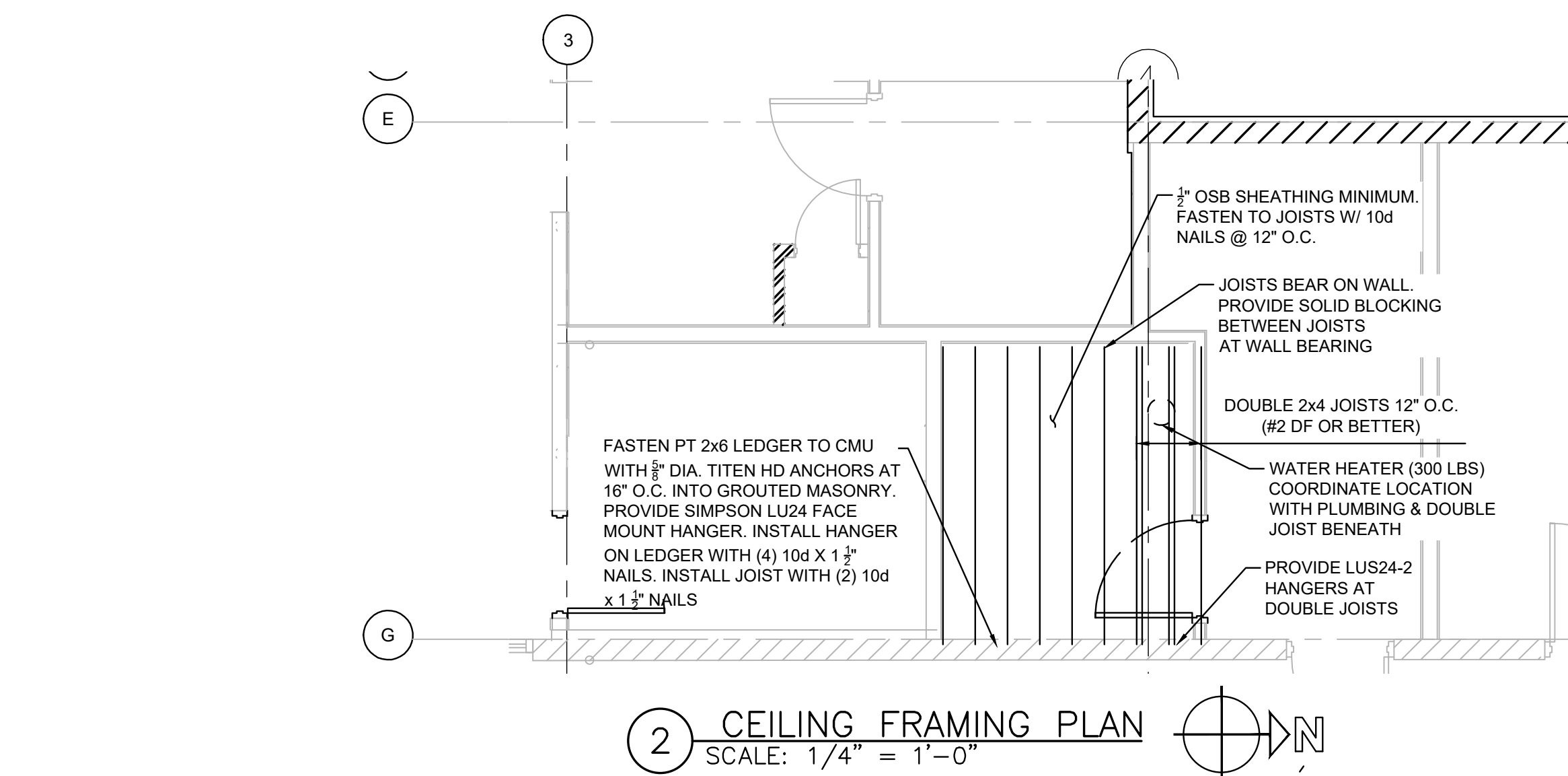
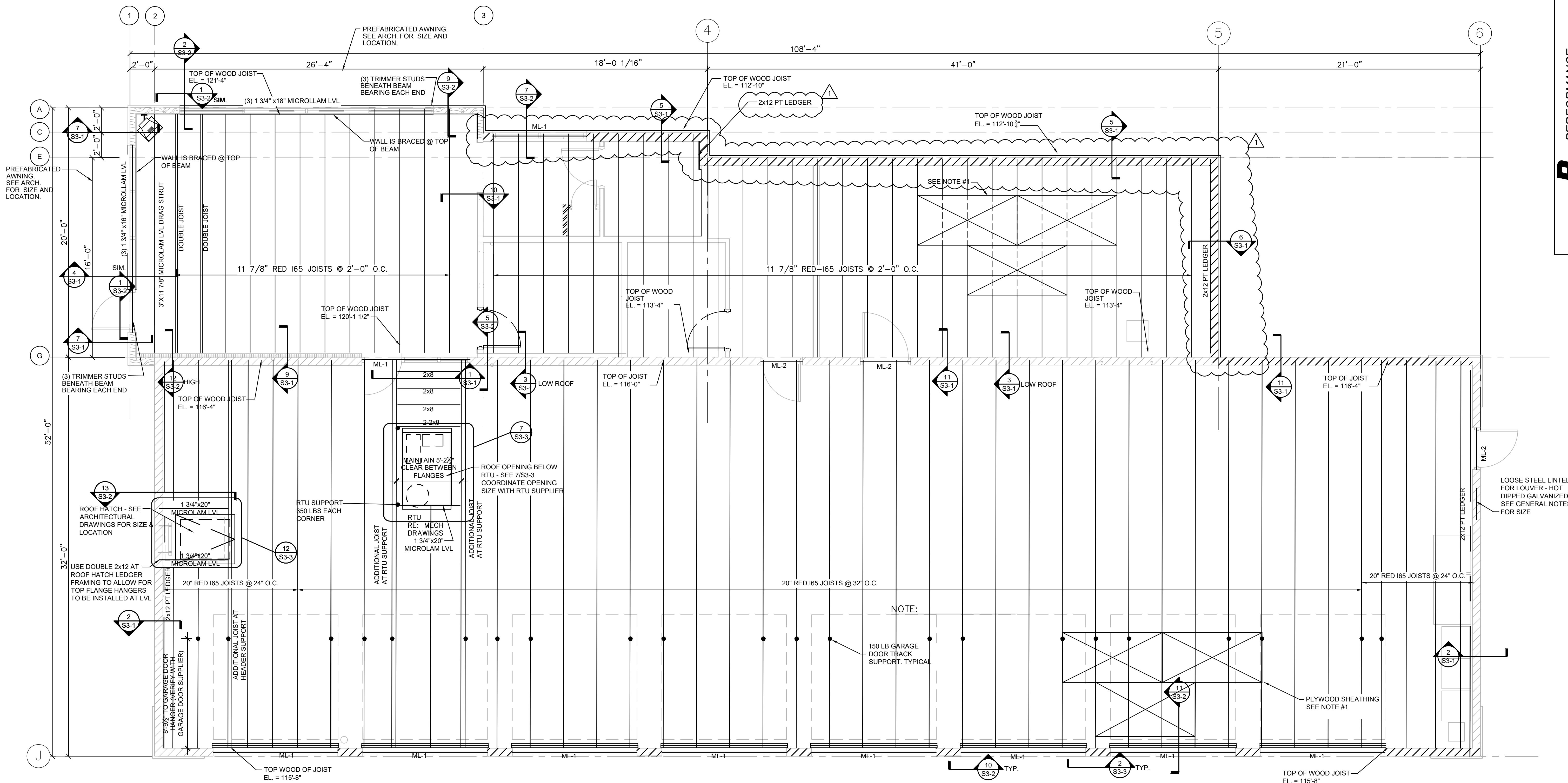


45 SPYGLASS DRIVE
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SHEET

S2-0

FOUNDATION PLAN



1 ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"

SHEET NOTES:

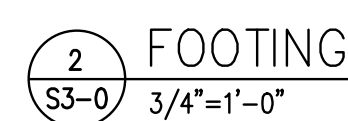
- DIAPHRAGM SHALL BE A MINIMUM OF 3/4" PLYWOOD SHEATHING WITH A SPAN RATING OF 48/24, EXTERIOR GRADE AND SHALL BE FASTENED TO STRUCTURE AS FOLLOWS:
 - 10d NAILS x 1 1/2"
 - SEE S1-0 FOR NAILING PATTERN OF ROOF DIAPHRAGM
 - FIELD NAILING = 12" O.C.
 - BLOCK EDGES OF PLYWOOD IN ROOF DIAPHRAGM AS SHOWN ON S1-0. BLOCKING SHALL BE A MINIMUM OF 2X NOMINAL THICKNESS
- ML-# INDICATES MASONRY LINTEL TYPE. SEE DETAIL 11/S3-2.
- UNLESS OTHERWISE DETAILED, CONNECTIONS FOR ITEMS HUNG FROM JOISTS (FANS, HEATERS, GARAGE DOORS, ETC.) SHALL BE DESIGNED AND INSTALLED BY THE GENERAL CONTRACTOR AND THEIR RESPECTIVE TRADES. THIS LOAD SHALL NOT EXCEED 250 LBS PER JOIST.
- ATTACH 2x12 LEDGER PIECES TO CMU WALL W/ (2) 3/4" DIA. ANCHORS @ 24" O.C., EMBED 6" MIN. FASTENERS SUCH AS NAILS, SCREWS, AND ANCHORS SHALL BE HOT DIPPED GALVANIZED WHEN CONNECTING TO OR ANCHORING PRESSURE TREATED LUMBER.
- ATTACH 3 1/2x20 LVL LEDGER TO CMU WALL W/ 3/4" DIA. A36 ANCHOR BOLTS @ 16" O.C. (HORZ.) TOP AND BOTTOM, 12" O.C. (VERT.). EMBED 6" MIN. PROTECT JOIST HANGERS FROM CONTACT WITH TREATED LEDGER VIA MOISTURE BARRIER (GRACE VYCORP DECK PROTECTOR), WHERE ANCHOR BOLTS ARE LOCATED, THE CELLS OF THE CLAY MASONRY UNITS ARE TO BE FULLY GROUTED. FASTENERS SUCH AS NAILS, SCREWS, AND ANCHORS SHALL BE HOT DIPPED GALVANIZED WHEN CONNECTING TO OR ANCHORING PRESSURE TREATED LUMBER.
- SEE DETAIL 7/S3-3 FOR ROOF OPENINGS & PENETRATIONS.
- WOOD WALLS SHALL BE 2x6 DOUGLAS FIR NO. 2 STUD. STUD SPACING SHALL BE 16" O.C. UNLESS NOTED OTHERWISE.
- ALL WOOD JOISTS SHALL BE DESIGNED TO SUPPORT THE LOADS INDICATED ON SHEET S1-0.

REVISION	DATE	COMMENTS
1	07.01.24	FOR BLDG. DEPT. SUBMITTAL
	07.23.24	ADDENDUM 1

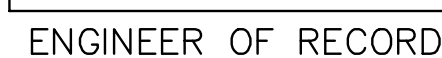
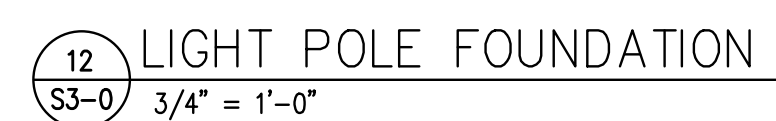
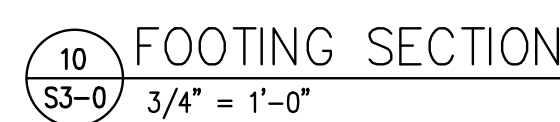
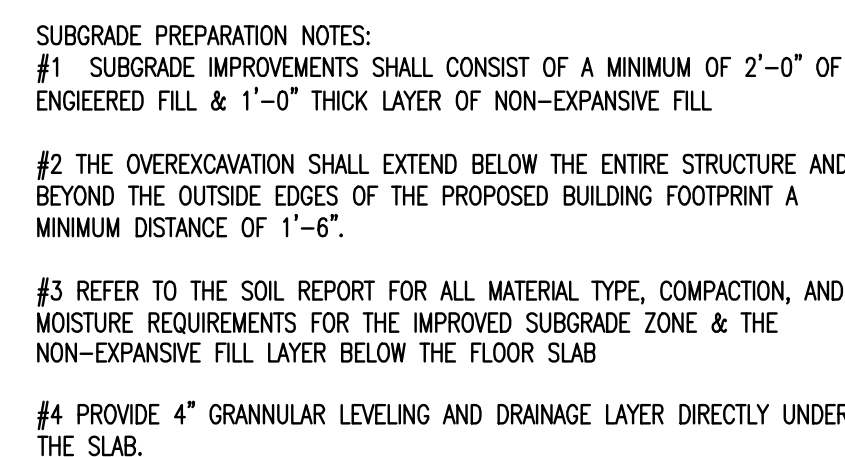
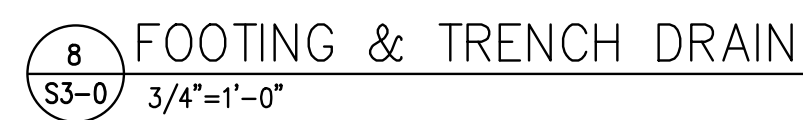
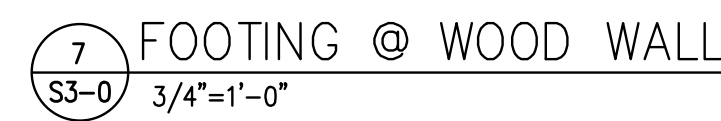
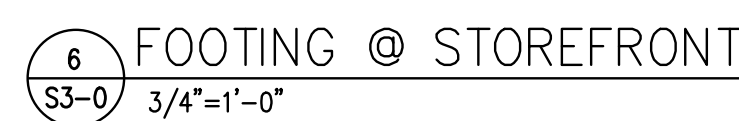
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CLIENT JOB #:
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CHECKED BY: TAS
DATE OF ISSUE: 05.13.24



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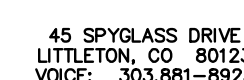
5 FOUNDATION SECTION
S3-0 3/4"=1'-0"



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	07.01.24	FOR BLDG. DEPT. SUBMITTAL
1	07.23.24	ADDENDUM 1

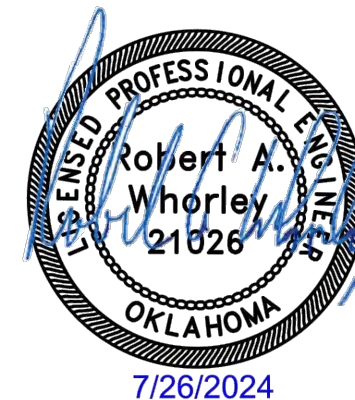
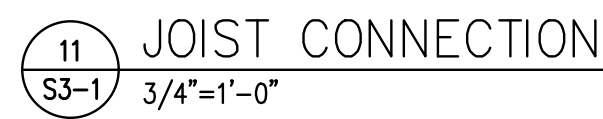
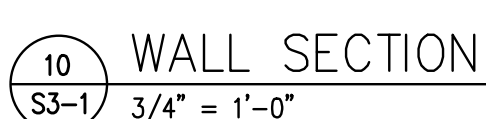
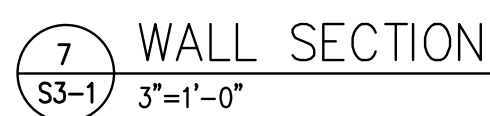
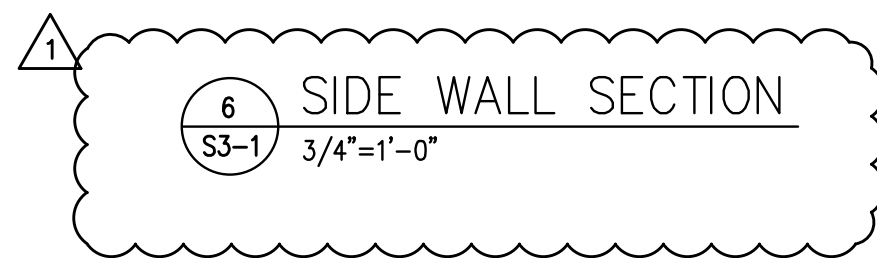
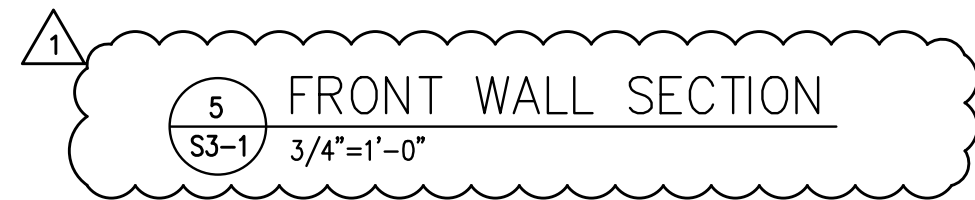
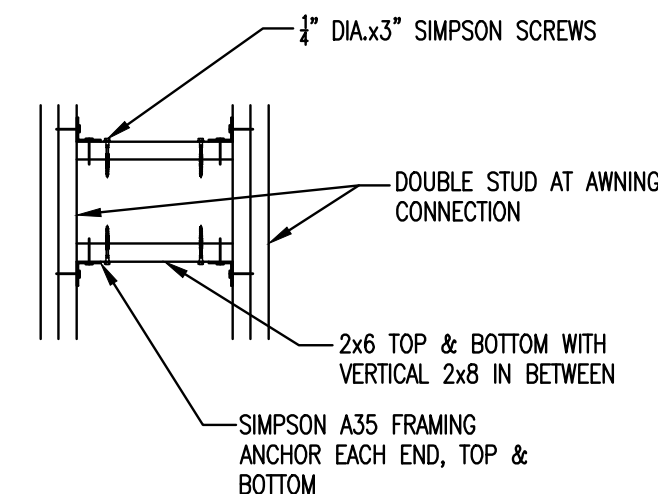
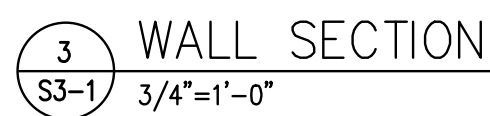
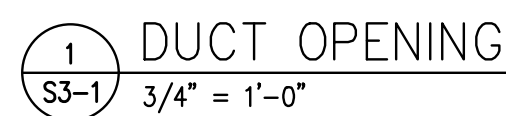
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CLIENTJOB #:	-
DRAWN BY:	SLM
CHECKED BY:	TAS
DATE OF ISSUE:	05.13.24



SHEET

S3-0

SECTIONS AND DETAILS



REVISION	DATE	COMMENTS
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1	07.23.24	ADDENDUM 1

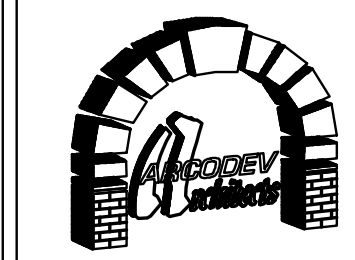
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CLIENTJOB #:

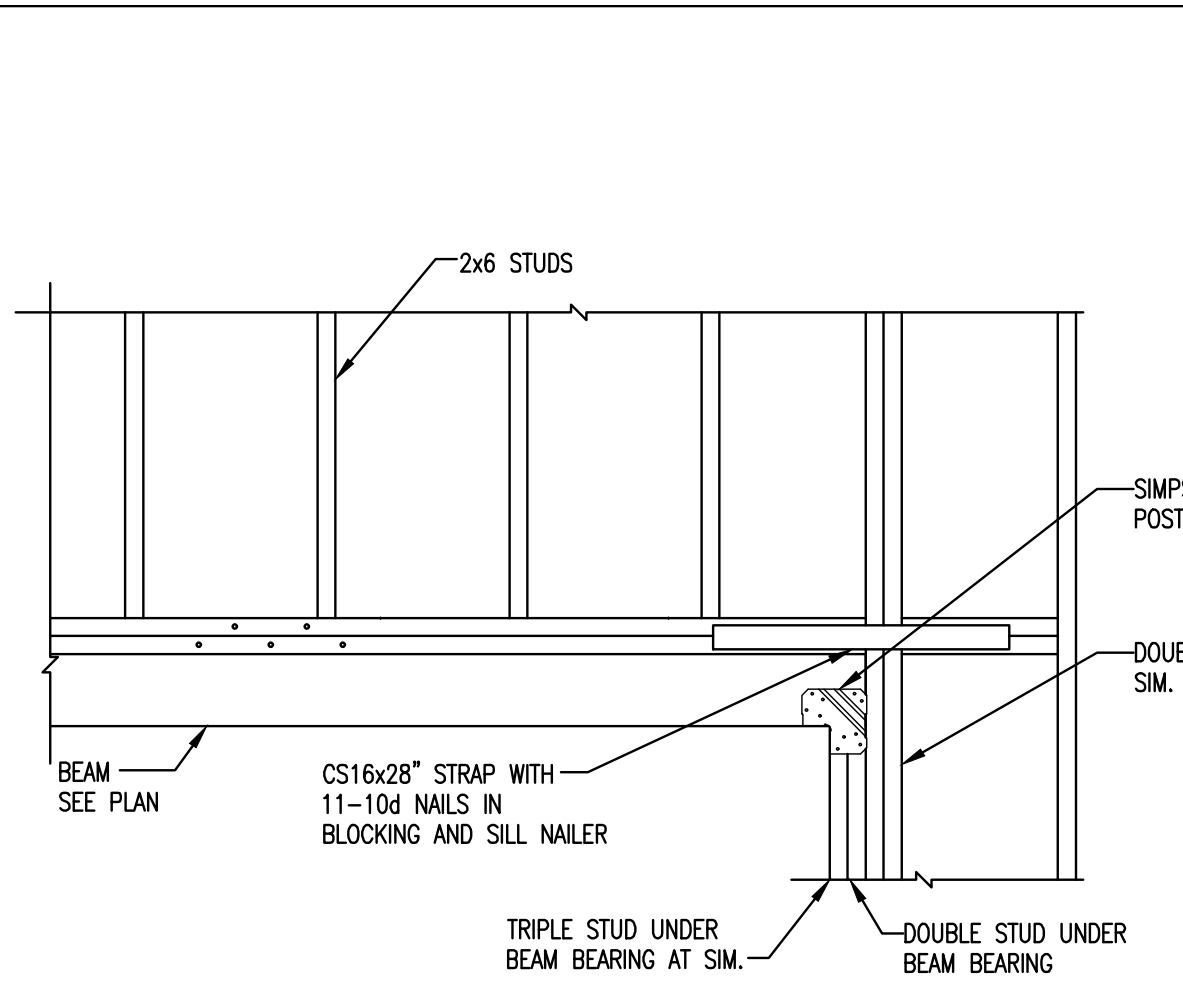
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CHECKED BY: _____ TA _____

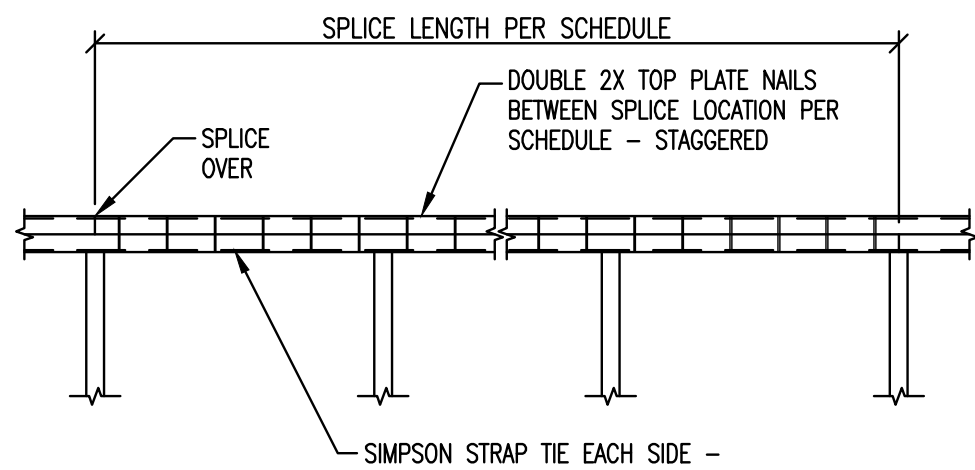
DATE OF ISSUE: 05.13.2



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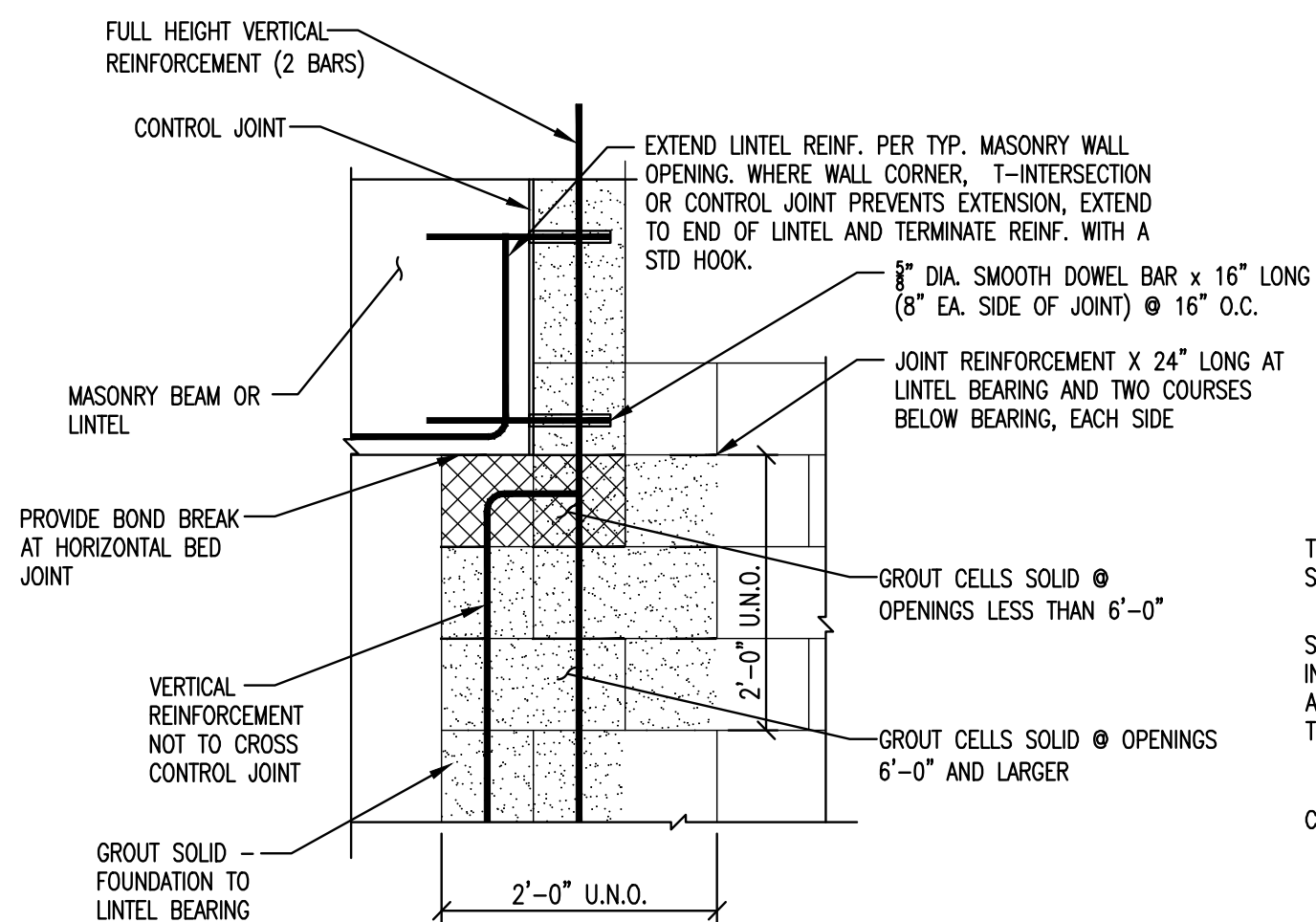
1
S3-2
3/4\"=1'-0\"



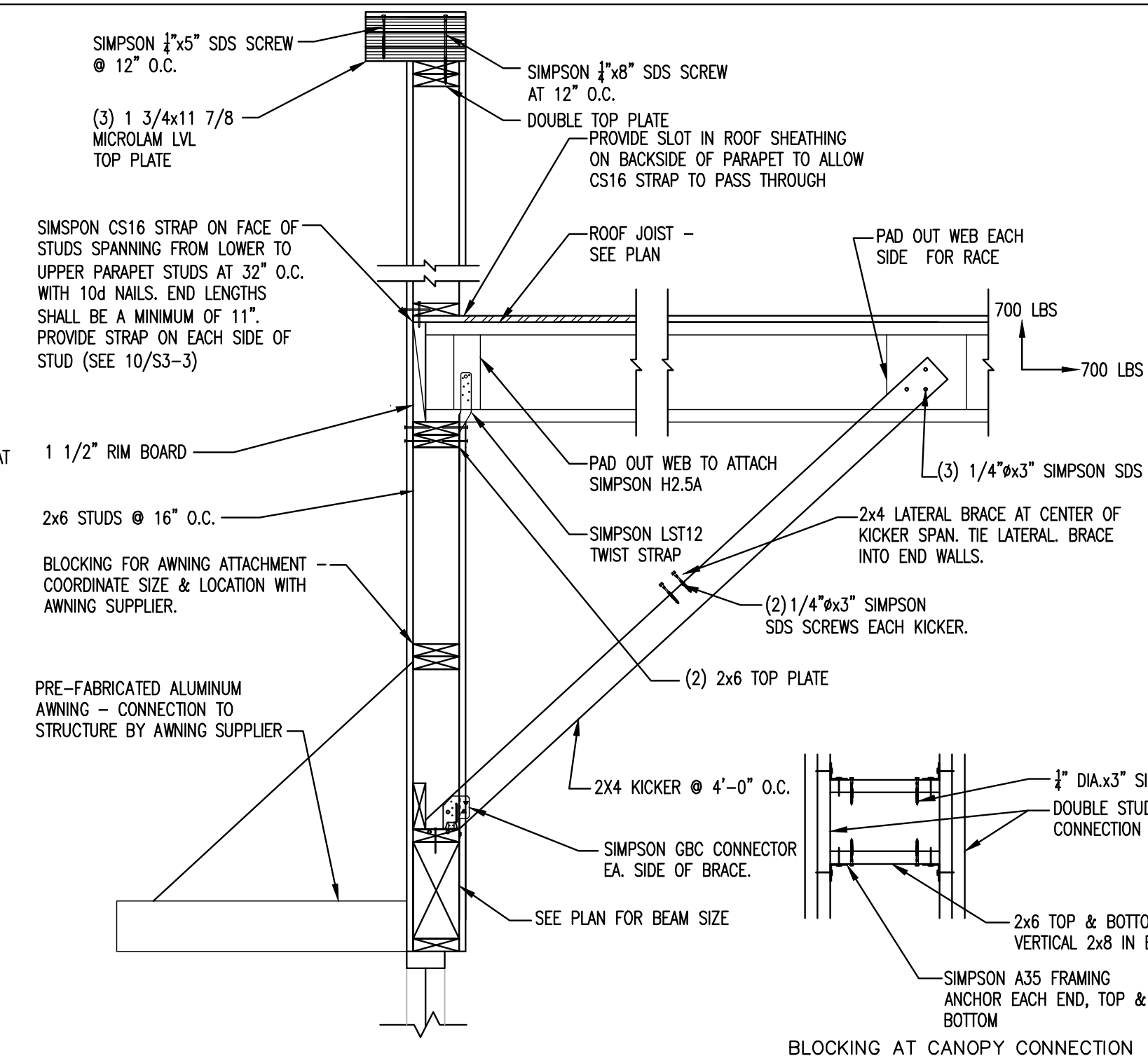
LENGTH OF WALL (BETWEEN CORNERS)	SPLICE LENGTH (MINIMUM)	NAILS ALONG SPLICE LENGTH
OVER 30'	4'-0"	18-16d
OVER 20'	2'-8"	10-16d
OVER 10'	1'-4"	6-16d
LESS THAN 10'	1'-4"	4-16d

NOTE:
1. DO NOT SPLICE TOP PLATES WITHIN 6'-0" OF ENDS OF WOOD STRUCTURAL PANEL SHEAR WALLS.
2. THIS DETAIL APPLIES TO ALL EXTERIOR WALLS AND INTERIOR WALLS. SIMPSON STRAP TIE NEED NOT BE APPLIED TO INTERIOR WALLS.
3. PROVIDE SIMPSON CMST 14 ON EACH SIDE OF TOP PLATE AT SPLICE LOCATION. EXTEND STRAP 34" BEYOND END OF PLATE SPLICE. INSTALL PER MANUFACTURER'S RECOMMENDATIONS.

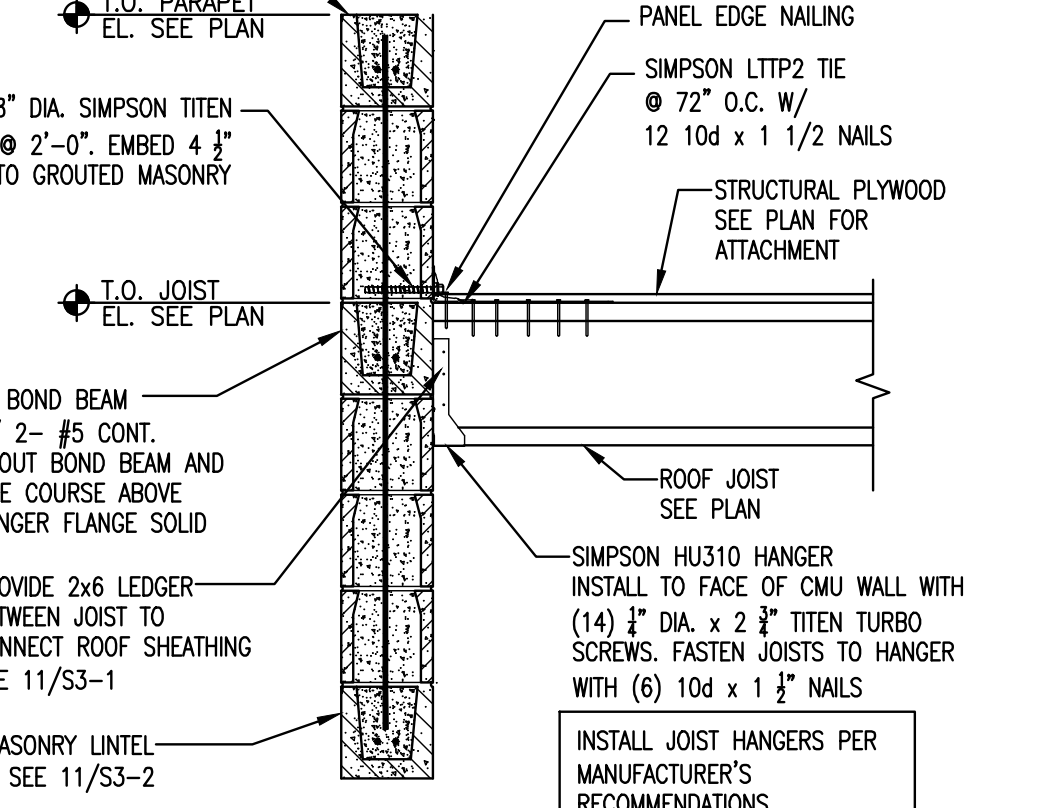
6
S3-2
3/4\"=1'-0\"



10
S3-2
3/4\"=1'-0\"



2
S3-2
3/4\"=1'-0\"

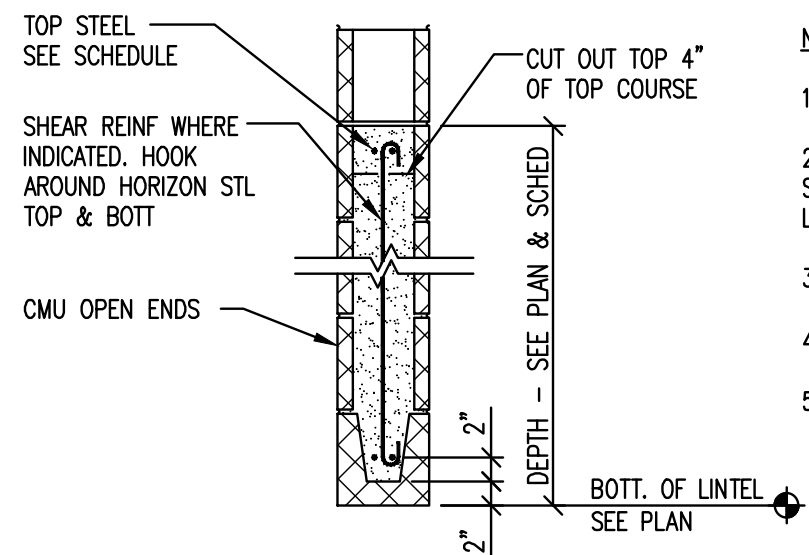


7
S3-2
3/4\"=1'-0\"

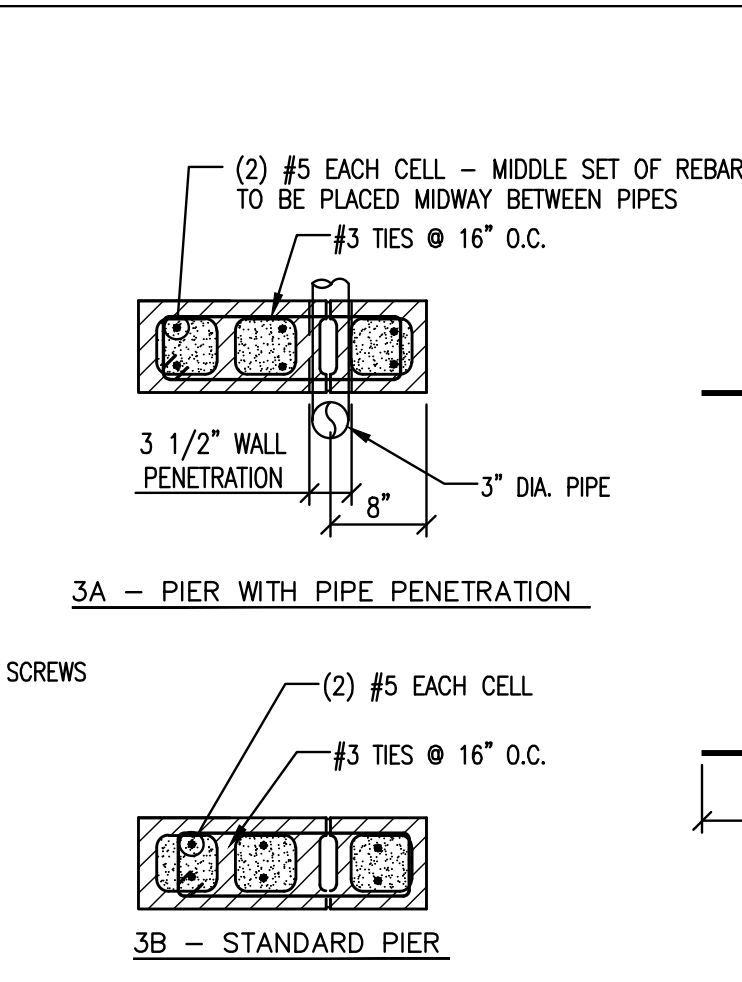
MASONRY LINTEL SCHEDULE

MARK	DEPTH	BOND BM REINF	SHEAR REINF	REMARKS
ML-1	2'-8"	2-#5 TOP & BOT	-	8" CMU
ML-2	1'-4"	2-#5 TOP & BOT	-	8" CMU

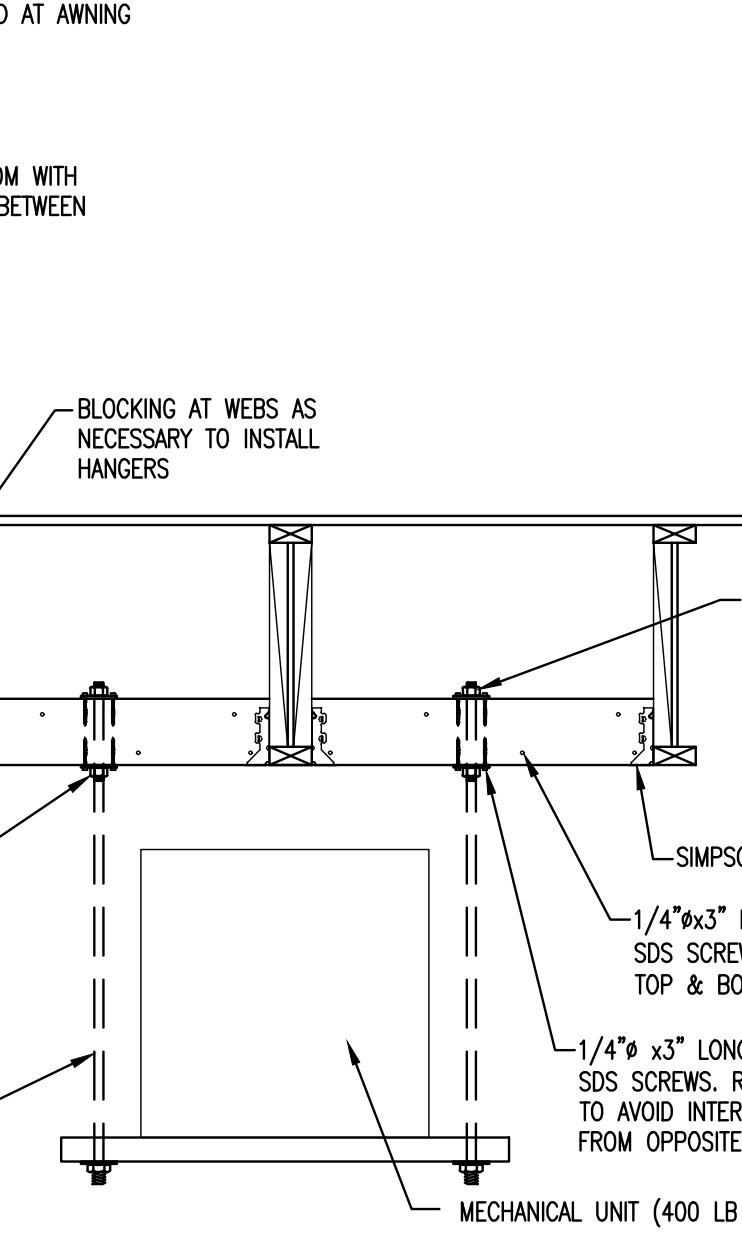
NOTE:
1. USE LINTEL ML-2 AT ALL OPENINGS LESS THAN 4'-0" WIDE UNLESS NOTED OTHERWISE.
2. BOND BEAM REINFORCING SHALL BE CONTINUOUS WITHOUT SPLICES.
3. MASONRY LINTELS SHALL BE GROUTED SOLID.



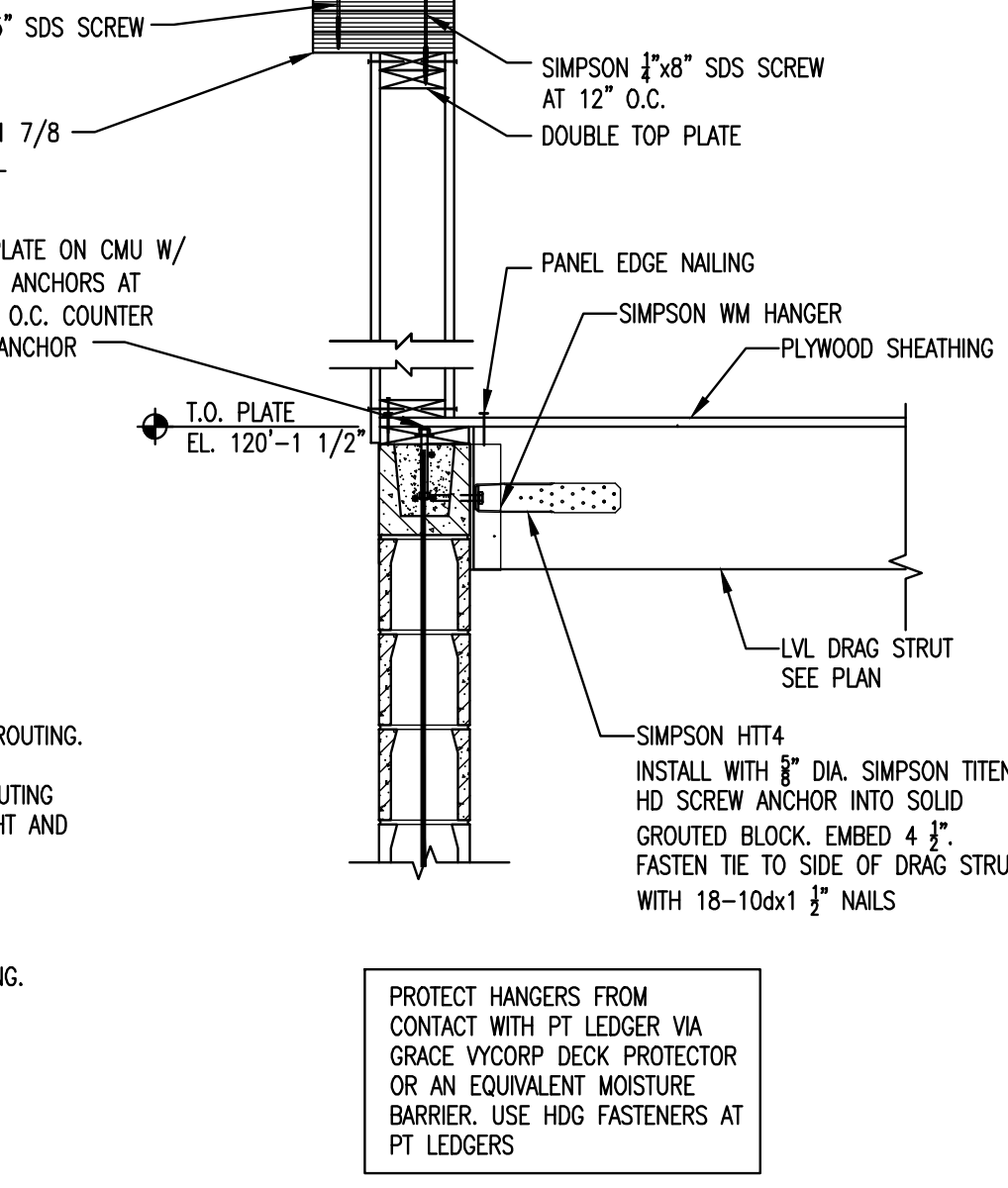
11
S3-2
3/4\"=1'-0\"



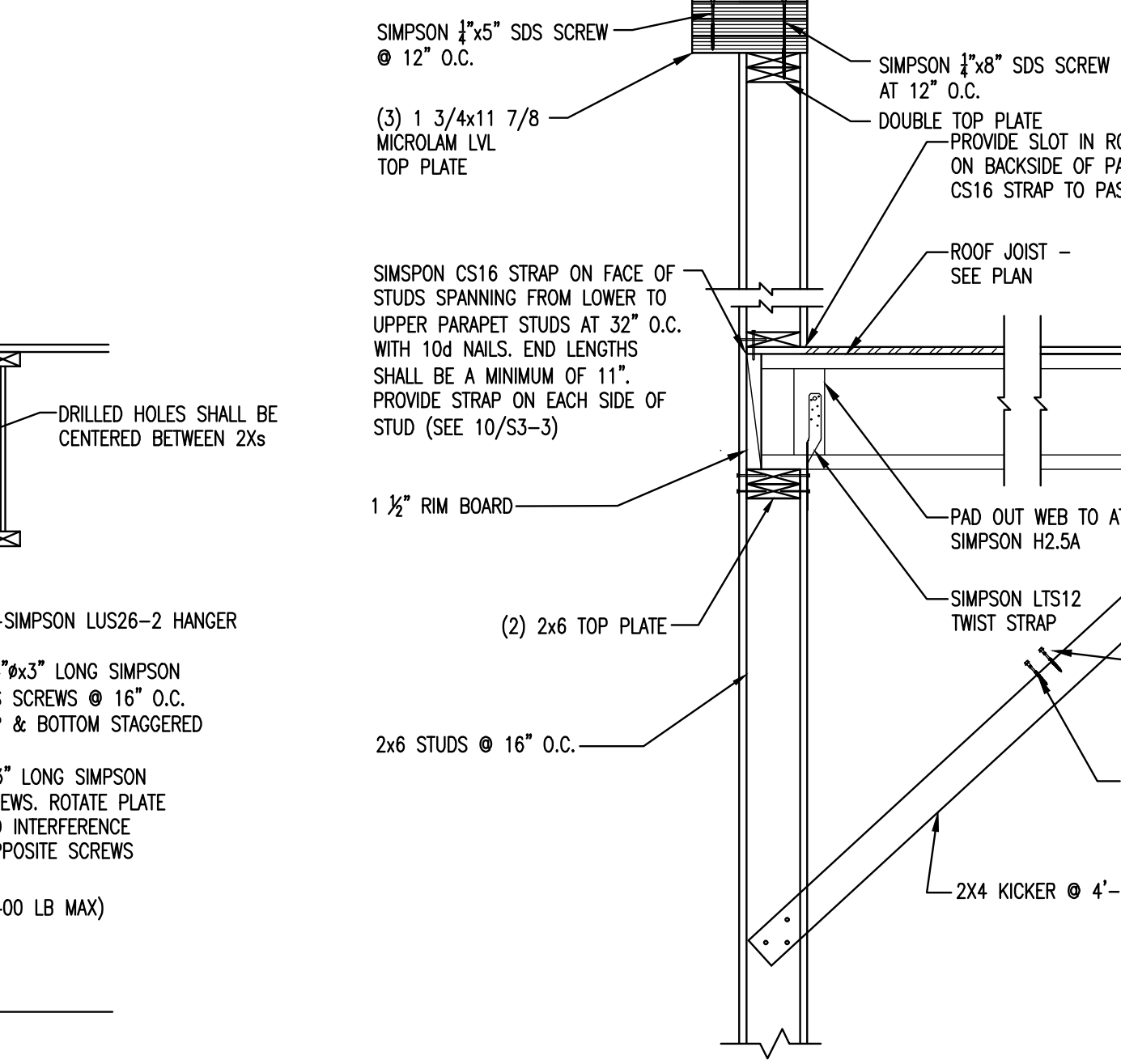
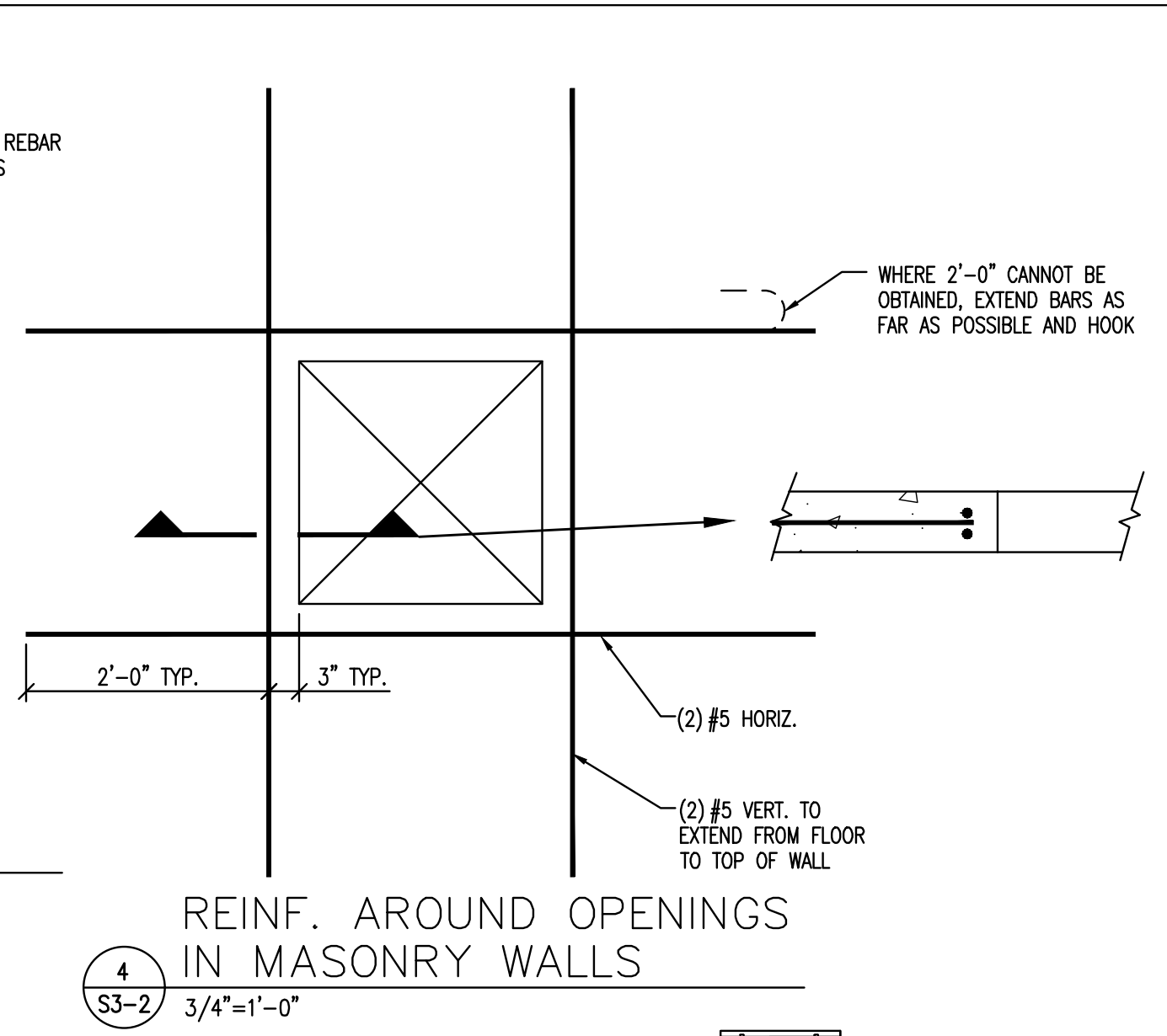
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S3-2
3/4\"=1'-0\"



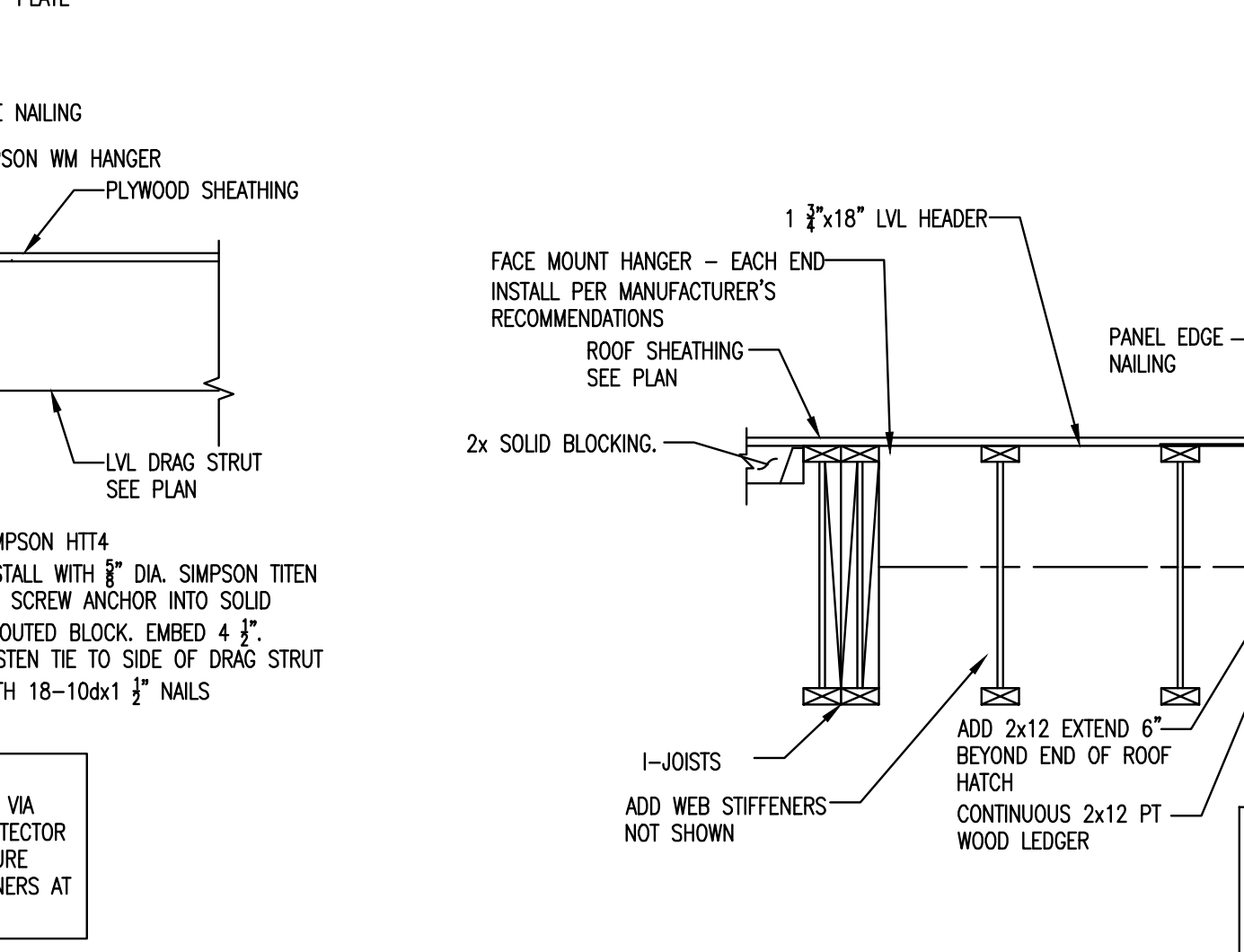
8
S3-2
3/4\"=1'-0\"



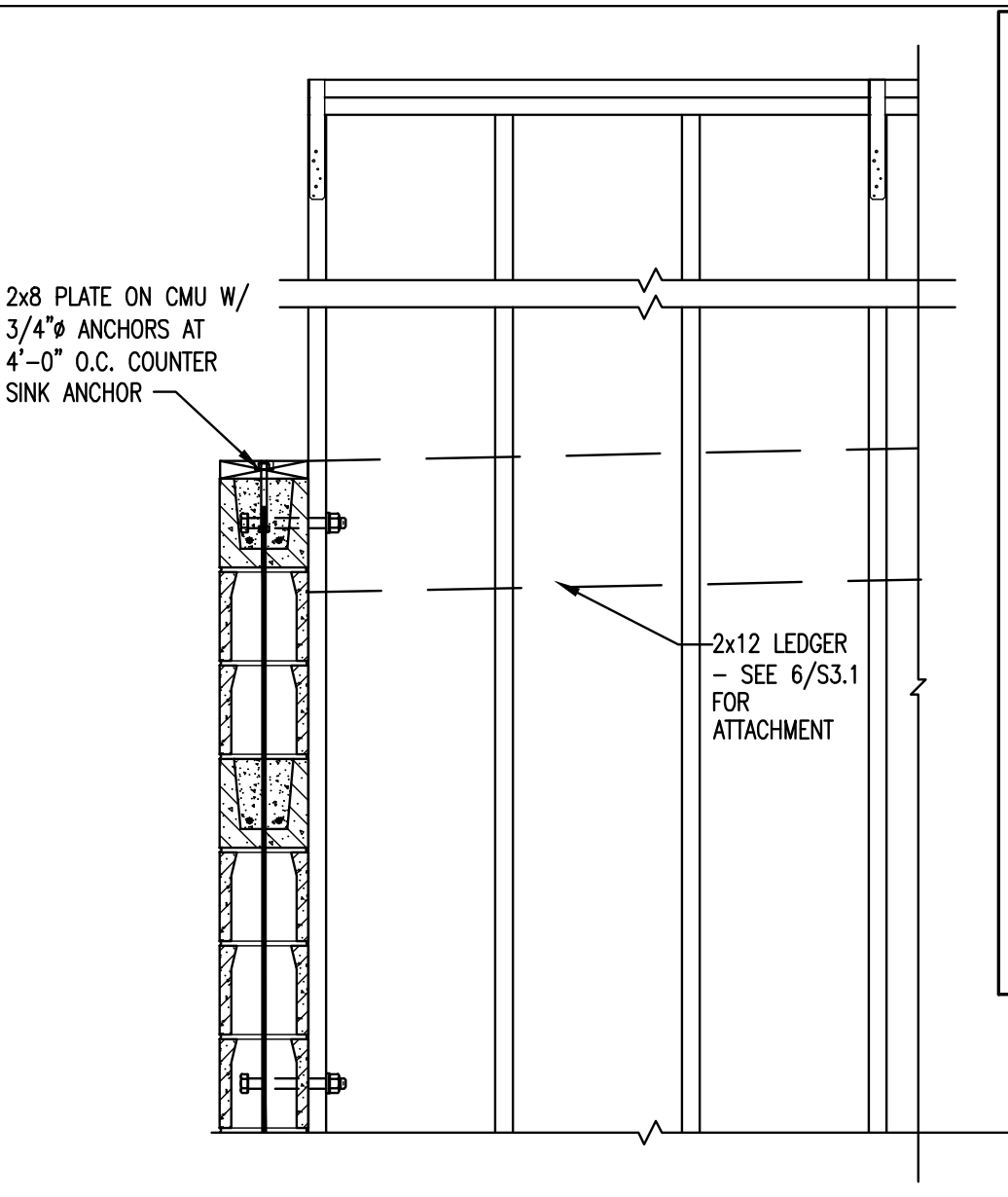
12
S3-2
3/4\"=1'-0\"



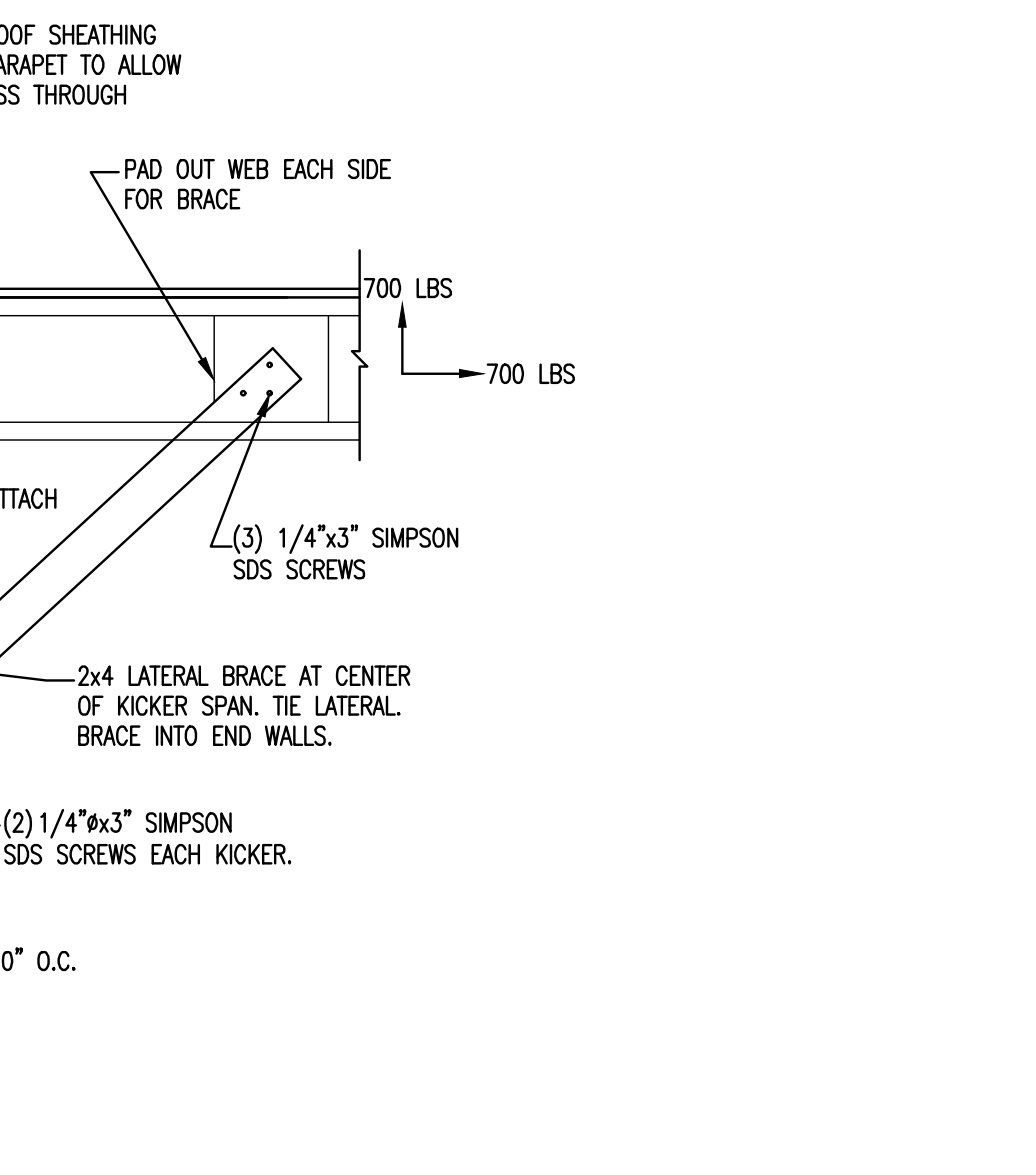
9
S3-2
3/4\"=1'-0\"



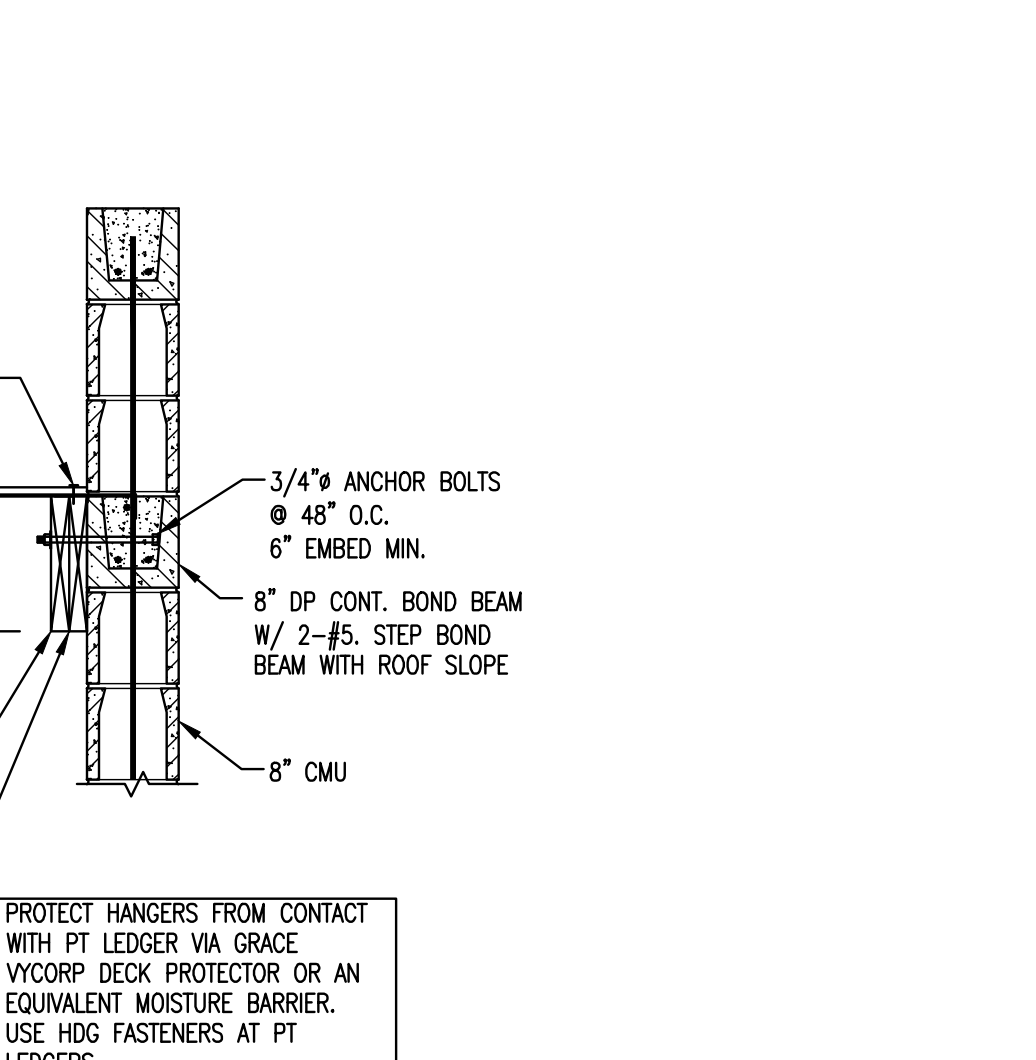
13
S3-2
3/4\"=1'-0\"



5
S3-2
3/4\"=1'-0\"



9
S3-2
3/4\"=1'-0\"



13
S3-2
3/4\"=1'-0\"

PERFORMANCE Engineering
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NE 064605
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BRAKES PLUS
2505 MAIN STREET
NORMAN, OKLAHOMA

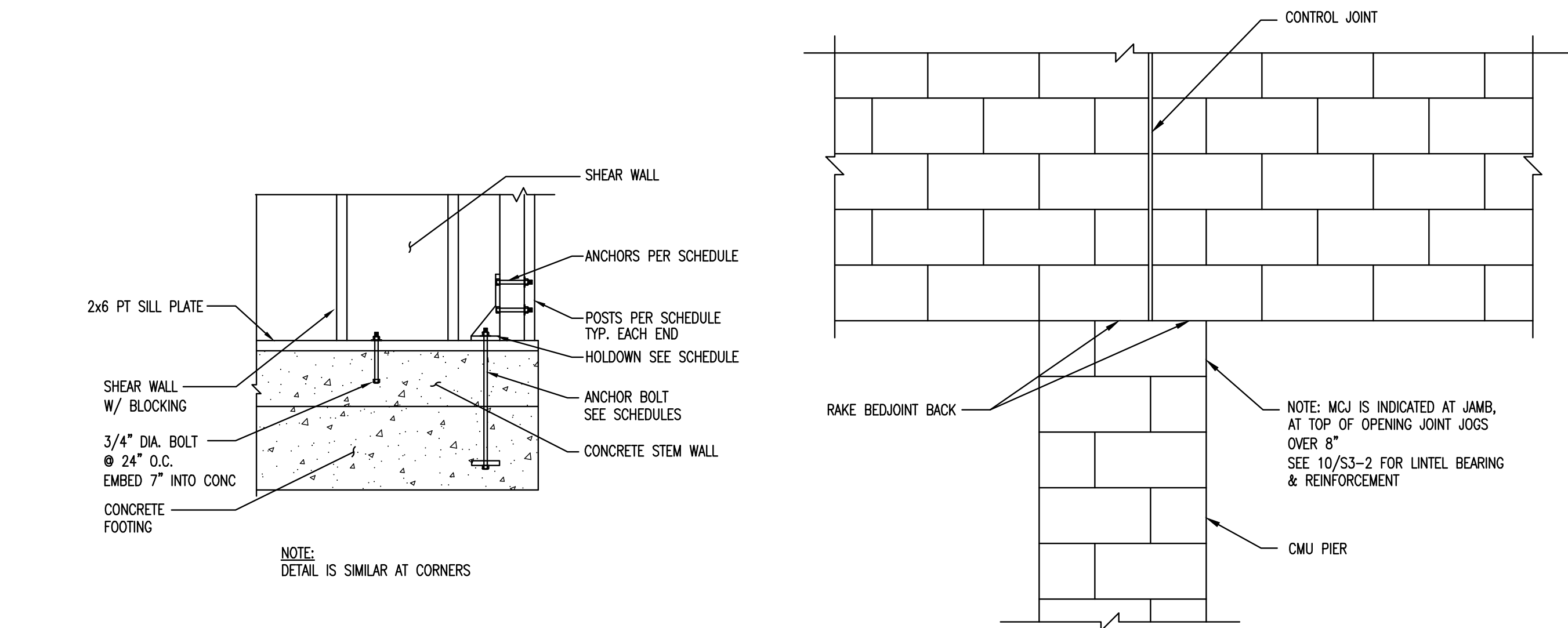
Robert A. Whorle
LICENSED PROFESSIONAL ENGINEER
OKLAHOMA
7/26/2024

REVISION	DATE	COMMENTS
	07.01.24	FOR BLDG. DEPT. SUBMITTAL
1	07.23.24	ADDENDUM 1

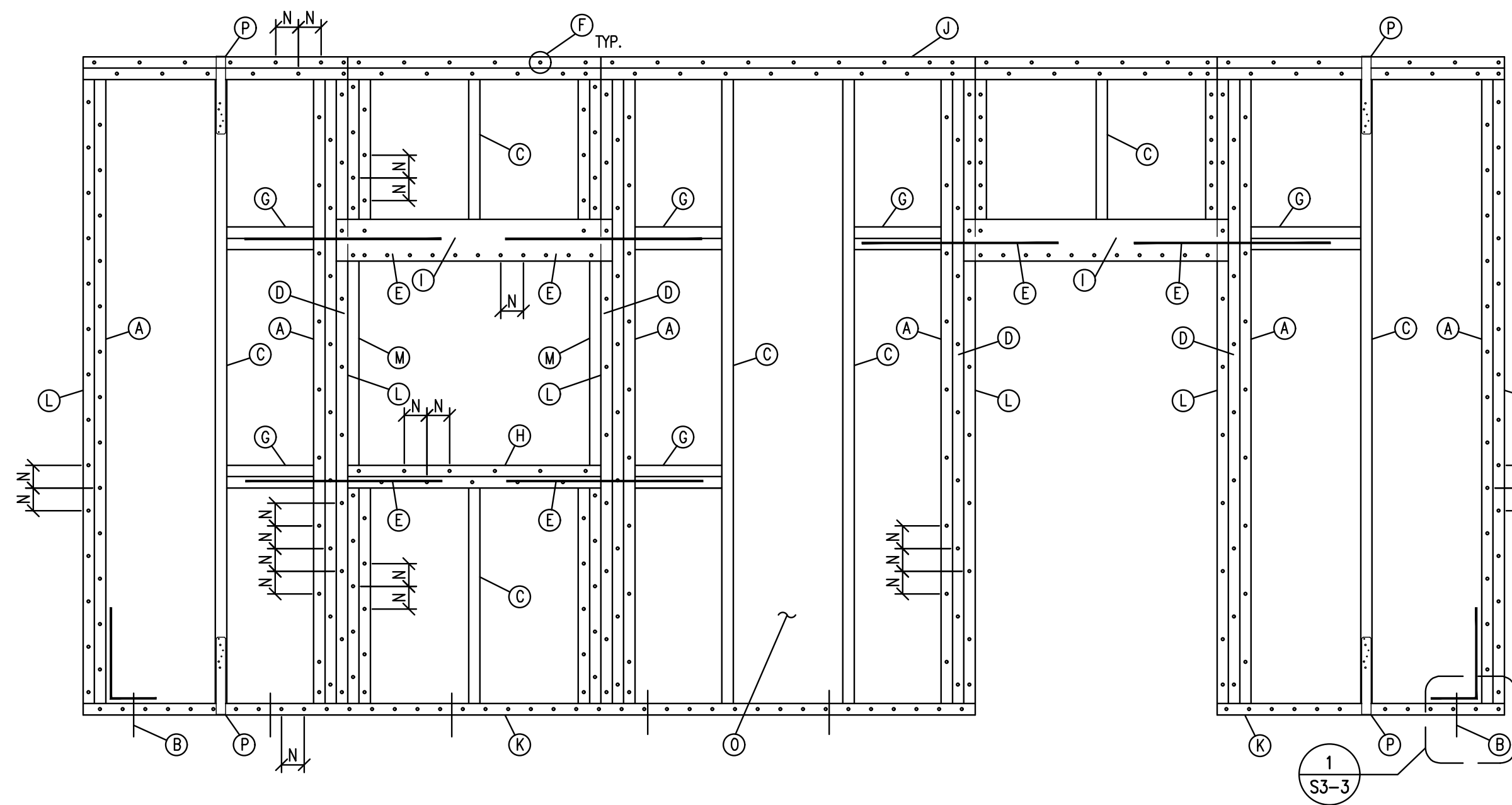
ARCOWE JOB # _____
CLIENT JOB # _____
DRAWN BY: SLW
CHECKED BY: TAS
DATE OF ISSUE: 05.13.24

ARCOWE
45 SPYGLASS DRIVE
LITTLETON, CO 80123
VOICE: 303.881-8925

SHEET
S3-2
SECTIONS AND DETAILS



1 FOOTING @ COLUMN
S3-3 3/4"=1'-0"

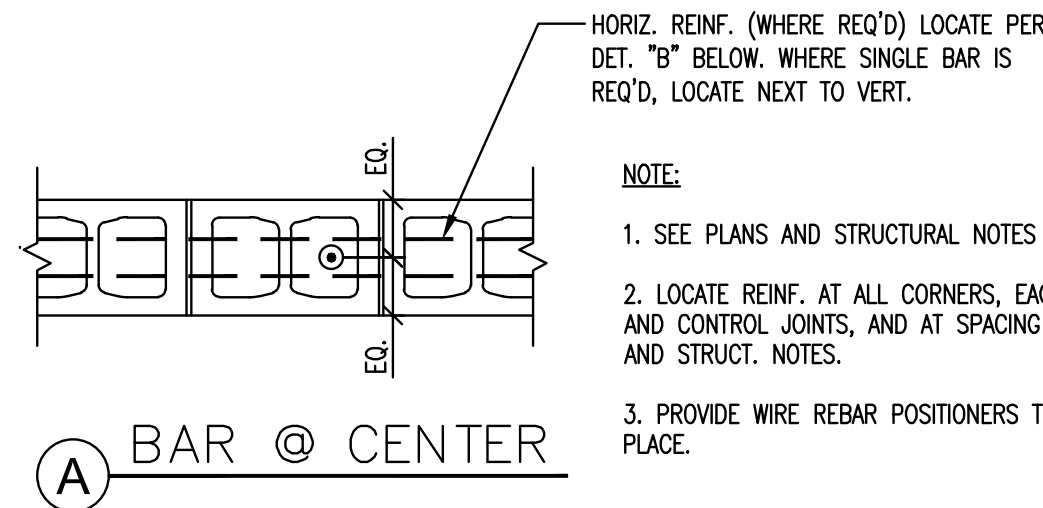


2 CONTROL JOINT @ PIER
S3-3 3/4"=1'-0"

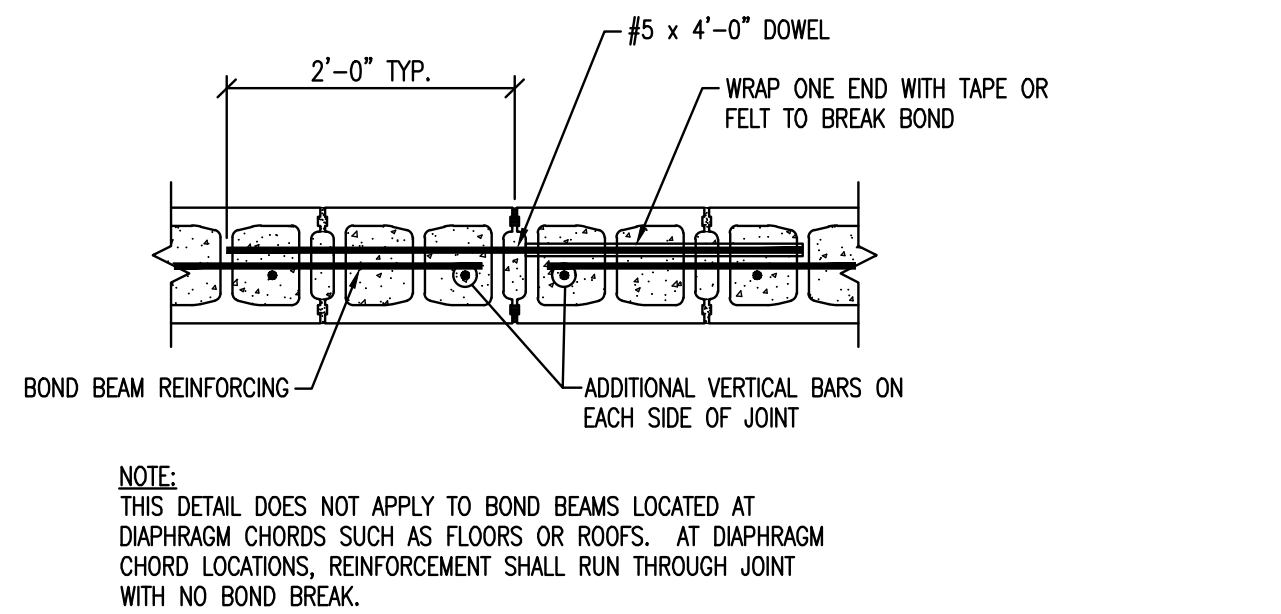
MASONRY REINF. SPLICE TABLE								
BAR SIZE	6" BLOCK	8" BLOCK		10" BLOCK		12" BLOCK		REMARKS
	BAR @ CL	BAR @ CL	BAR @ EDGE	BAR @ CL	BAR @ EDGE	BAR @ CL	BAR @ EDGE	
#4	2'-1"	1'-9"	2'-7"	2'-1"	2'-5"	2'-1"	2'-4"	-
#5	3'-3"	2'-2"	4'-0"	2'-7"	3'-10"	2'-7"	3'-7"	-
#6	-	3'-6"	8'-2"	4'-4"	7'-8"	4'-4"	7'-3"	-
#7	-	5'-0"	-	5'-1"	10'-5"	5'-1"	9'-10"	-

NOTE: WHEN REQUIRED SPLICE LENGTH EXCEEDS 4'-0" USE HIGH LIFT GROUTING WITH NO SPLICES OR USE MECHANICAL TENSION SPLICES WITH LOW LIFT GROUTING.

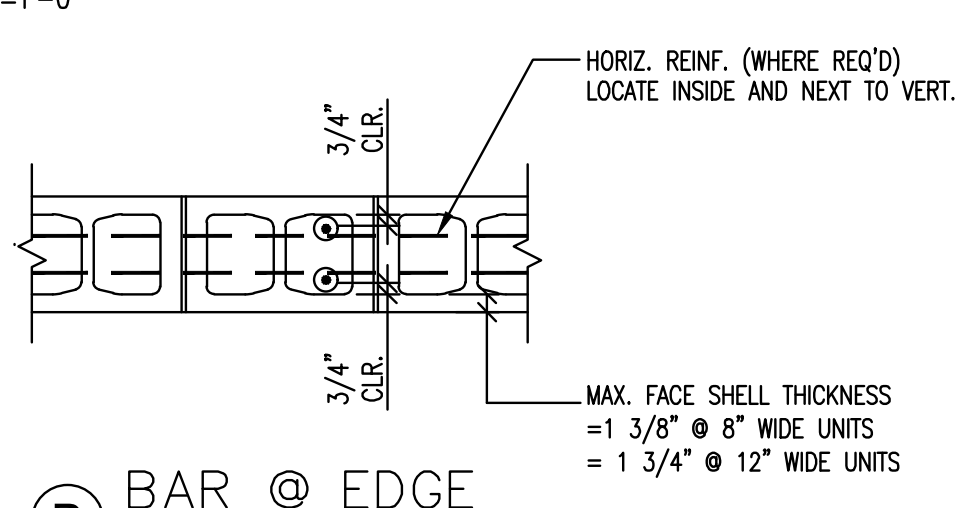
3 MASONRY REINFORCEMENT SPLICE & DEVELOPMENT LENGTH
S3-3 3/4"=1'-0"



A BAR @ CENTER

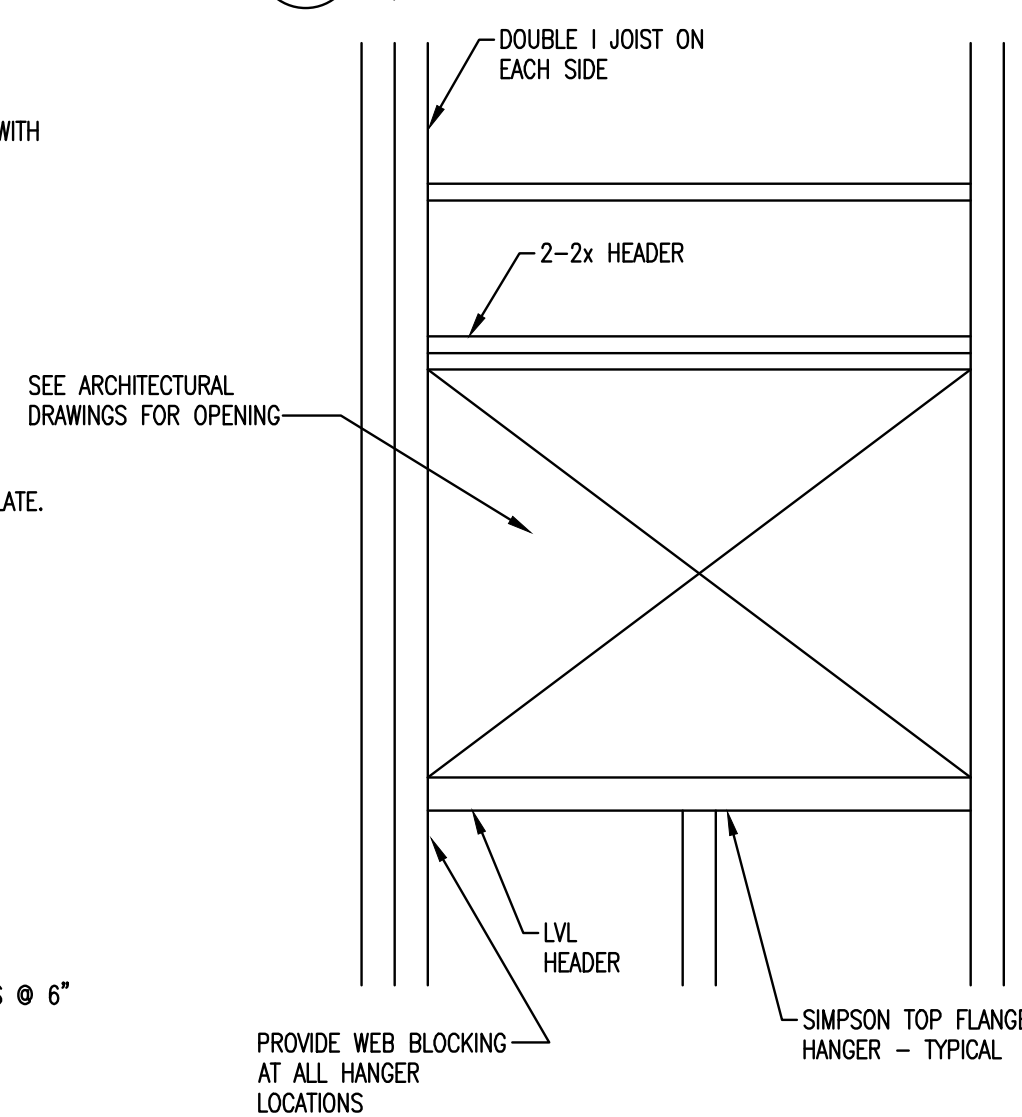


4 MASONRY CONTROL JOINT
S3-3 3/4"=1'-0"

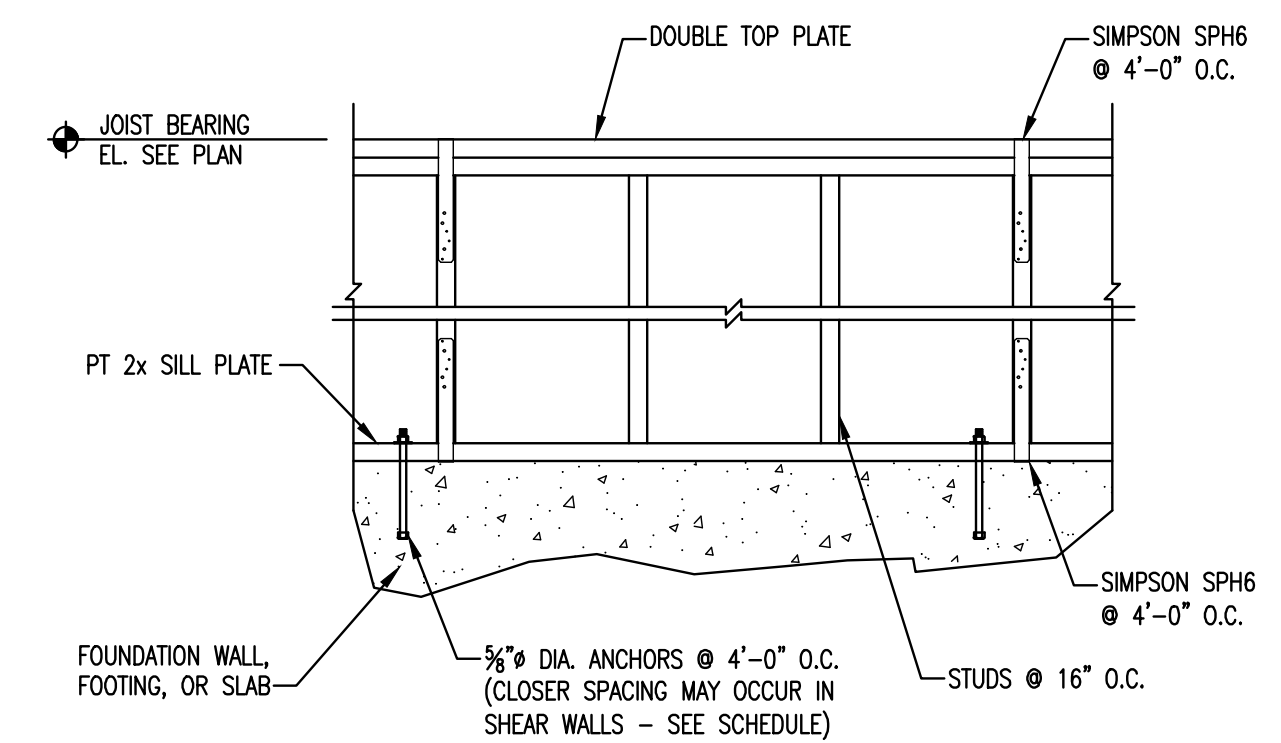


B BAR @ EDGE

6 TYP. MASONRY WALL REINF. PLACEMENT
S3-3 3/4"=1'-0"

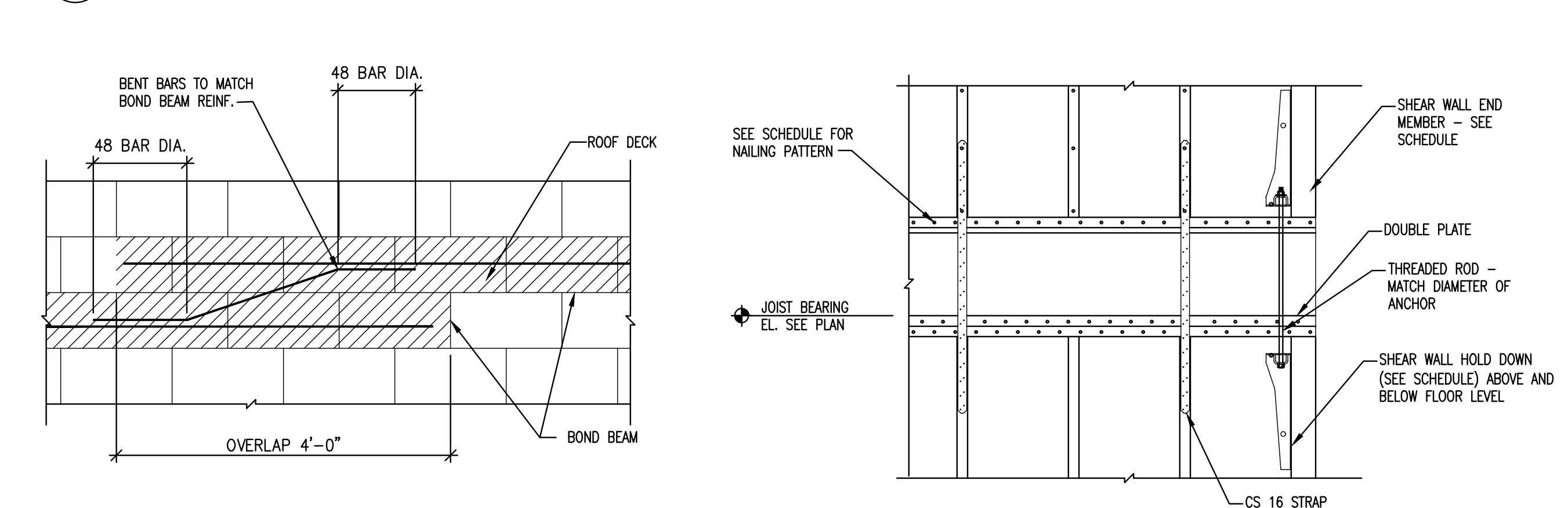


7 WOOD ROOF OPENING PLAN
S3-3 3/4" = 1'-0"

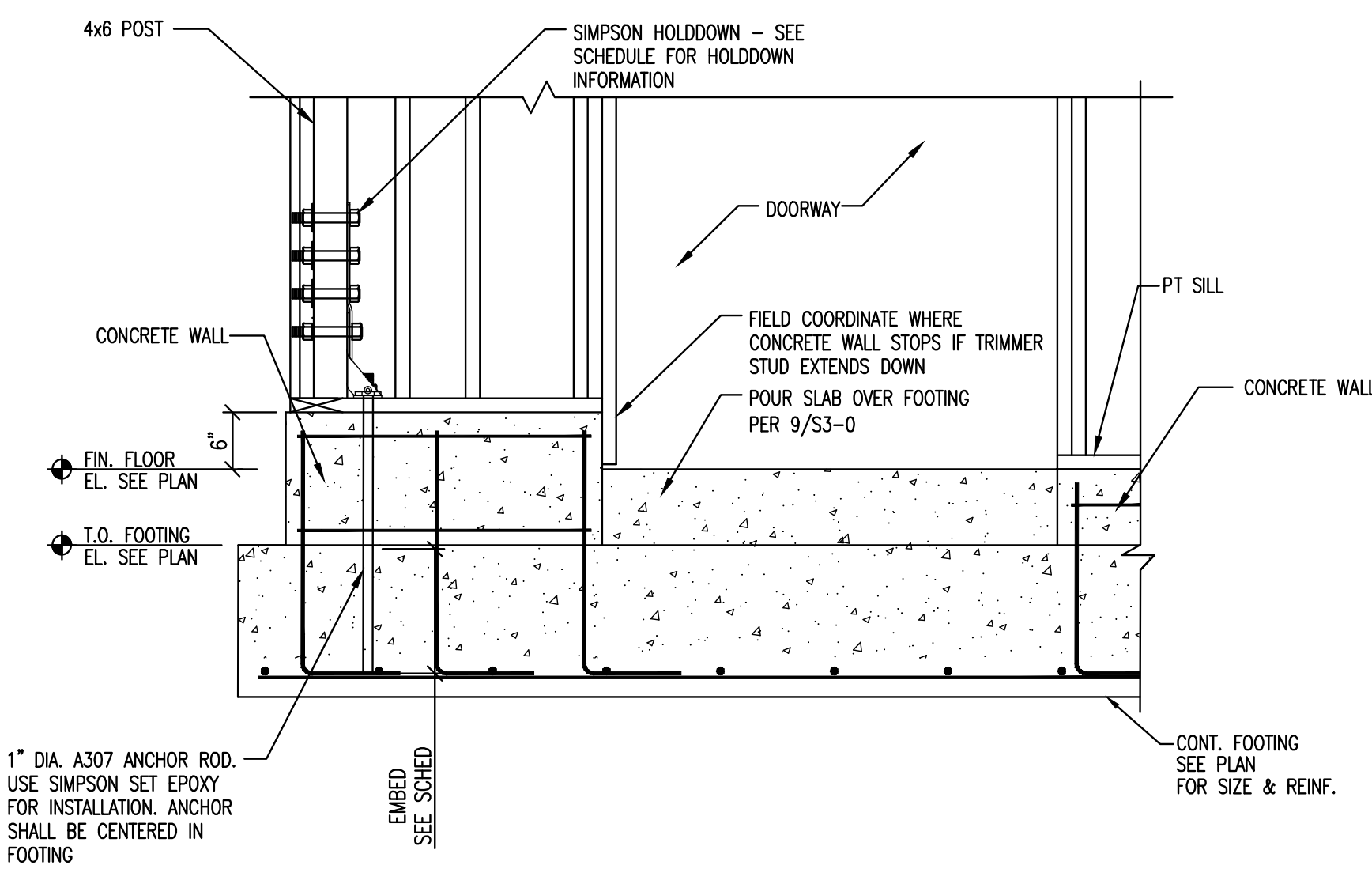


8 TYPICAL WALL FRAMING
S3-3 3/4"=1'-0"

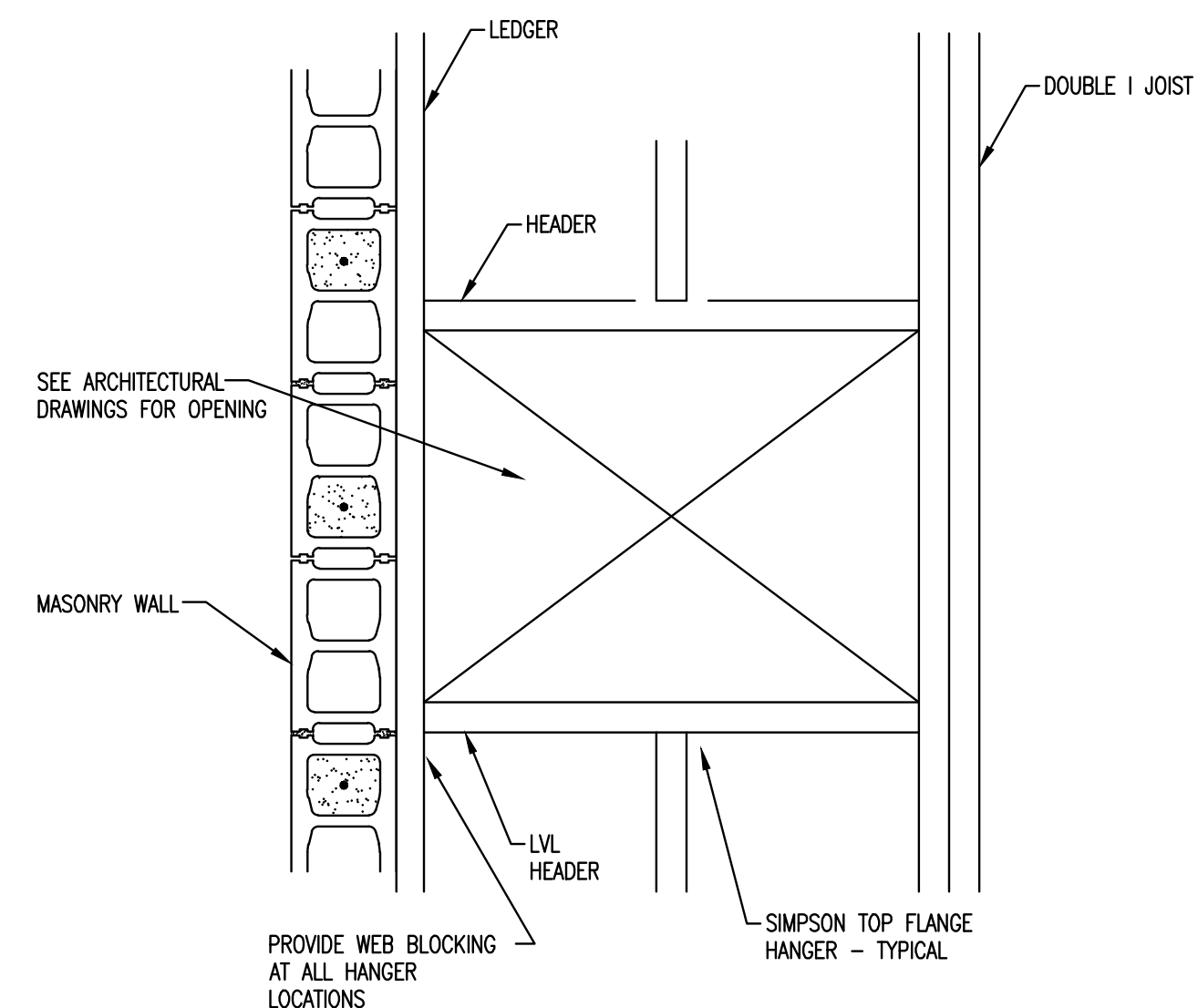
5 SHEAR WALL CONSTRUCTION
S3-3 3/4"=1'-0"



10 TWO LEVEL SHEAR WALL CONNECTION
S3-3 3/4" = 1'-0"



11 FOOTING SECTION
S3-3 NTS



12 ROOF ACCESS OPENING
S3-3 3/4" = 1'-0"

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1	07.01.24	FOR BLDG. DEPT. SUBMITTAL
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ARCDEV JOB #:
CLIENT JOB #:
DRAWN BY: SLW
CHECKED BY: TAS
DATE OF ISSUE: 05.13.24